

# A comparison between advanced hybrid machine learning algorithms and empirical equations applied on abutment scour depth prediction

Khabat Khosravi\*<sup>1</sup>, Zohreh Sheikh Khozani<sup>2</sup>, Luca Mao<sup>3</sup>

1- Department of Watershed Management Engineering, Ferdowsi University of Mashhad, Mashhad, Iran.

2- Institute of Structural Mechanics, Bauhaus Universität Weimar, 99423 Weimar, Germany.

3- School of Geography, University of Lincoln, Lincoln, UK.

\*Corresponding author: K. Khosravi ([Khabat.khosravi@gmail.com](mailto:Khabat.khosravi@gmail.com))

## Abstract

Complex vortex flow patterns around bridge piers, especially during floods, cause scour process that can result in the failure of foundations. Abutment scour is a complex three-dimensional phenomenon that is difficult to predict especially with traditional formulas obtained using empirical approaches such as regressions. This paper presents a test of a standalone Kstar model with five novel hybrid algorithm of bagging (BA-Kstar), dagging (DA-Kstar), random committee (RC-Kstar), random subspace (RS-Kstar), and weighted instance handler wrapper (WIHW-Kstar) to predict scour depth ( $ds$ ) for clear water condition. The dataset consists of 99 scour depth data from flume experiments (Dey and Barbhuiya, 2005) using abutment shapes such as vertical, semicircular and 45° wing. Four dimensionless parameter of relative flow depth ( $h/l$ ), excess abutment Froude number ( $Fe$ ), relative sediment size ( $d_{50}/l$ ) and relative submergence ( $d_{50}/h$ ) were considered for the prediction of relative scour depth ( $ds/l$ ). A portion of the dataset was used for the calibration (70%), and the remaining used for model validation. Pearson correlation coefficients helped deciding relevance of the input parameters combination and finally four different combinations of input parameters were used. The performance of the models was assessed visually and with quantitative metrics. Overall, the best input combination for vertical abutment shape is the combination of  $Fe$ ,  $d_{50}/l$  and  $h/l$ , while for semicircular and 45°

28 wing the combination of the  $Fe$  and  $d_{50}/l$  is the most effective input parameter combination. Our  
29 results show that incorporating  $Fe$ ,  $d_{50}/l$  and  $h/l$  lead to higher performance while involving  $d_{50}/h$   
30 reduced the models prediction power for vertical abutment shape and for semicircular and  $45^\circ$   
31 wing involving  $h/l$  and  $d_{50}/h$  lead to more error. The WIHW-Kstar provided the highest  
32 performance in scour depth prediction around vertical abutment shape while RC-Kstar model  
33 outperform of other models for scour depth prediction around semicircular and  $45^\circ$  wing.

34 Keywords: Abutment, scour depth, machine learning, empirical models.

35

## 36 **1. Introduction**

37 Bridges are critical infrastructures, and the failure of their piers can lead to severe economical  
38 and social consequences. The most common failure mode for bridges over rivers is generally due  
39 to intense local scouring around their piers. Therefore, a reliable estimation of abutment scour  
40 and its disruptive effects are crucially important to design these infrastructures as overestimating  
41 or underestimating scour can result in higher construction cost and abutment failure, respectively  
42 (Azamathulla et al., 2009; Cardoso and Bettess, 1999). Bridge abutments change the local flow  
43 pattern and generally cause the formation of a three-part separation zone around a bridge pier.  
44 The pressure gradient due to the presence of the pier forces a down-flow that causes the scouring  
45 in front of the pier. This leads to the generation of a so-called horseshoe vortex which facilitate  
46 further the scouring in front of pier (Melville and Coleman, 2000; Török et al., 2014). The shear  
47 stresses at the upstream face of an abutment due to the principal vortices also facilitate secondary  
48 vortices. In addition, unsteady shear layers that are generated at the pier rotate in vertical axes  
49 (wake vortices) as small eddies. Furthermore, bow-waves can also contribute to the scouring  
50 process. A combination of these vortexes eventually lead to scour holes around piers of bridges

51 (Laursen and Toch, 1956; Liu et al. 1961; Kwan, 1988; Hosseini et al., 2016), and detailed  
52 descriptions on abutment scour depth process are widely available in the literature (e.g., Dey,  
53 1997; Dey and Barbhuiya, 2005; Gazi et al., 2019; Kothiyari et al., 1992; Melville and Raudkivi,  
54 1977; Melville and Sutherland, 1988; Moonen and Allegrini, 2015; Namaee and Sui, 2019;  
55 Raudkivi and Ettema, 1983).

56 A local scour can be created in conditions in which no sediments are proceeding from upstream  
57 reaches (i.e., clear water, or no sediment feeding from upstream) or in more natural conditions in  
58 which the flow approach the pier with sediments (i.e., sediment is fed from upstream reaches). A  
59 wide range of experimental and field studies investigated the process of scour depth around  
60 bridges under clear water conditions, due to the simplicity of this condition. Dey and Lambert  
61 (2005) conducted experiments under clear water conditions and investigated the evolution over  
62 time and the equilibrium conditions of scour depth for three different shapes of short abutments  
63 (vertical wall, semicircular, and 45° wing wall) using both uniform and nonuniform sediments.  
64 They applied the concept of mass conservation of sediment to derive numerical equations to  
65 calculate the scour depth evolution over time. Oliveto and Hager (2002) conducted a further set  
66 of experiments and proposed an equation that allows to calculate the scour depth around both  
67 piers and abutments that worked reasonably well when applied to other experimental datasets  
68 too, especially for rectangular cross section and uniform distribution of roughness. Amini et al.  
69 (2012) further revealed that the pile spacing, diameter, and the submerge ratio are three  
70 important parameter which can affect the scour depth. Although many flume experiments have  
71 been carried out to support scour depth modeling (Ataie-Ashtiani et al., 2010; Ataie-Ashtiani and  
72 Beheshti, 2006; Singh et al., 2020; Yang et al., 2020), this approach suffers from scale effect  
73 issues which can have an impact on the applicability of the results. Also, the experimental

74 approach is costly and time-consuming. Flume experimental data are generally used to derive  
75 empirical equations based on regressions but this approach, albeit practical, are too simplistic to  
76 represent the complexity of flows around a bridge piers (Azamathulla et al., 2009). Numerical  
77 investigations of scour depth have been attempted using SSIIM models (Hamidi and  
78 Siadatmousavi, 2018; Jahangirzadeh et al., 2014), Smagorinsky subgrid model combined with a  
79 ghost-cell immersed boundary method (Kim et al., 2014), Virtual Flow Simulator (VFS-  
80 Geophysics) (Khosronejad et al., 2020), FLUENT (Yang et al., 2005), and FLOW-3D (Omara et  
81 al., 2019), but applications of these models are restricted due to the paucity of large  
82 experimentally and field measurement dataset for their calibration and validation. Although these  
83 models consider the physics of the scouring processes, their implementation is difficult, time  
84 consuming, and needs large and accurate datasets.

85 An alternative to traditional approaches is provided by the use of Artificial Intelligence (AI) as it  
86 is user-friendly, easy to perform, requires less data, is robust to missing data, and provides high  
87 accuracy to predict complex phenomena especially in engineering and geoscience fields.  
88 Artificial intelligence has the ability to train complex and hidden relationships between inputs  
89 and outputs without a detailed knowledge of the physics of the problem. Employing AI for  
90 predicting scour depth around different hydraulic structures has indeed been attempted in  
91 literature in the past decade (Ebtehaj et al., 2018; Guven and Azamathulla, 2012; Guven and  
92 Gunal, 2008; Najafzadeh et al., 2013a; Najafzadeh and Lim, 2015).

93 Artificial Neural Networks (ANN) is the traditional and most widely used algorithm for scour  
94 depth prediction (Amini et al., 2012; Kaya, 2010; Yazdandoost and Birgani, 2011). Three  
95 different ANN techniques as the Feed Forward Back Propagation (FFBP), Feed Forward  
96 Cascade Correlation (FFCC) and Radial Basis Function (RBF) were applied by Muzzammil

97 (2008) to estimate scour depth in clear-water condition for vertical wall abutments. In his study  
98 the input and output data were normalized (0 and 1) and the impact of dimensionless and  
99 dimensional inputs in modeling the scour depth was investigated. There were only a few later  
100 application of ANN models due to many critical disadvantages as the low speed convergence and  
101 poor generalization power (Choubin et al., 2018; Hooshyaripor et al., 2014). Also, the  
102 performances of the ANN model strongly depend of the extension of the dataset (Hooshyaripor  
103 and Tahershamsi, 2013). To overcome this issue, adaptive Neuro-Fuzzy Inference System  
104 (ANFIS) was developed as an ensemble of ANN and fuzzy logic. Bateni and Jeng (2007)  
105 employed an ANFIS method to simulate the scour depth. Hosseini et al. (2016) compared the  
106 prediction power of ANFIS, ANN and multiple nonlinear regression (MNLR) for scour depth  
107 prediction and finally stated that the ANFIS model has a higher prediction capability than ANN  
108 and MNLR models. Still, the ANFIS model suffers from determining the weights in a  
109 membership function, which affect significantly the result. Abd El-Hady Rady (2020) reported  
110 on the superiority of genetic programming (GP) over ANFIS algorithm for scour depth around  
111 bridge pier. Support Vector Machine (SVM) is another type of neuron-based model which was  
112 successfully applied in scour depth prediction. Parsaie et al. (2019) observed that the SVM  
113 model has a higher prediction capability for scour depth prediction than ANN and ANFIS  
114 algorithm. Ahmad et al., (2018) revealed that SVM is sensitive to hyper-parameter selection, and  
115 Najafzadeh et al. (2016) reported that ANFIS performed better than SVM and traditional existing  
116 equations. Further, the Group Method of Data Handling (GMDH) is a model which can  
117 automatically select the number of neurons and the network layers and allows to obtain a  
118 mathematical model in terms of polynomials for the target parameter. However, being a kind of  
119 neuron-based model, GMDH is sensitive to the extension of the dataset. Najafzadeh et al. (2013)

120 applied both GMDH and SVM approaches to a set of experimental data to predict scouring depth  
121 in four different shapes of abutments in both clear water and sediment feeding conditions. They  
122 used a backward path (BP) algorithm to design topology of the GMDH model in order to  
123 improve the performance of model and discovered that the BP-GMDH performed better than the  
124 SVM in both conditions. Scour depth around abutments was also predicted by using the pareto  
125 evolutionary structure of ANFIS network by Azimi et al. (2017; 2019). Using the dataset from  
126 Dey and Lambert (2005) they used a sensitivity analysis to rank the role of eleven different  
127 dimensional input variables which effect scour depth. Also, they compared their best developed  
128 model (i.e., ANFIS-GA/SVD 7) with other techniques employed previously (i.e., Azamathulla,  
129 2012; Moradi et al., 2019; Muzzammil, 2010; Najafzadeh et al., 2013b) and revealed that the  
130 ANFIS-GA/SVD model could provide more accurate results of scour depth in comparison with  
131 GEP, ANFIS–SC, ANFIS and GMDH models. Extreme Learning Machine (ELM) is another  
132 neuron-based algorithm with faster training phase and has been successfully applied in a  
133 different field of study. Ebtehaj et al. (2018) reported that the ELM model has a higher  
134 performance than ANN and SVM models to predict scour depth. In a recent study, Bonakdari et  
135 al. (2020) used the ELM technique to predict scour depth in clear-water condition considering  
136 four different nondimensional input parameters to estimate scour depth. About 11 different input  
137 combinations were tested to find the best one and they finally concluded that the model which  
138 contains all input variables allowed to obtain a better performance. They extended a matrix-  
139 based equation to calculate the scour depth, but their equation is highly complex and need large  
140 mathematic calculations. Because ELM is a version of the ANN model, its performance strongly  
141 depends on the extent of the dataset and is hampered by low performances with small datasets.

142 To overcome the shortcomings of the aforementioned traditional machine learning algorithm,  
143 different kind of data mining algorithms have been recently developed. These are tree-based  
144 [random forest (RF), random tree (RT), M5 prime (M5P), reduced error pruning tree (REPT)],  
145 rule-based [M5 Rule (M5R)], lazy-learn-base [Kstar, instance-based K-nearest neighbors (IBK),  
146 locally-weighted learning (LWL)], regression-based [sequential minimal optimization regression  
147 algorithm (SMO)] and ensemble-based [bagging (BA), random committee (RC), random  
148 subspace (RS)] algorithms. Some of these techniques operate for classification as well as for  
149 regression, based on the learner. The superiority of the RF algorithm over ANN and SVM in  
150 infiltration process prediction was reported by Sihag et al. (2020). Also, Yan et al. (2012) found  
151 that M5P algorithm had a higher prediction capability than ANN model for daily suspended  
152 sediment load prediction. Different metaheuristic algorithms were applied to solve the weakness  
153 of neuron-based models which suffer from determination of weights in membership function and  
154 operators. Khosravi et al. (2019a) compared predictive modeling of standalone new algorithms  
155 of M5P, RT, RF, REPT, Kstar with standalone and optimized ANFIS model using metaheuristic  
156 algorithms for reference evaporation prediction. They found that new a decision-tree based  
157 standalone model and Kstar models have higher performance than the ANFSI model, while  
158 optimized ANFIS models performed slightly better than standalone new models. Except for  
159 better prediction power and more flexibility, new algorithms require fewer parametric settings,  
160 making them more practical for real applications.

161 Sheikh Khozani et al. (2019) employed different standalone and a hybrid model to predict  
162 apparent shear stress in compound channels. They found that the BA-M5P model predicted the  
163 apparent shear stress with higher accuracy than standalone models. Khosravi et al. (2020)  
164 implemented four standalone algorithm of decision tree and four ensemble-based model using

165 BA algorithm for bedload transport rate prediction, and found that ensemble-based models  
166 predicted bedload with higher prediction accuracy. Similar observations have been reported by  
167 Bui et al. (2020b) and Khosravi et al. (2018).

168 The main objective of the present study is to predict abutment scour depth using a suite of new  
169 standalone and ensemble-based models. To meet the aim, standalone KStar algorithm are applied  
170 as a base model along with five novel ensemble-based models of BA-KStar, dagging (DA-  
171 KStar), RC-Kstar, RS-Kstar and Weighted Instance Handler Wrapper model (WIHW-Kstar).  
172 Finally, the results are compared with two traditional empirical equations (Dey and Barbhuiya,  
173 2005 and Muzzammil, 2010) as a benchmark.

## 174 **2. Methodology**

### 175 ***2.1. Identifying effective parameters***

176 Scour depth ( $d_s$ ) at abutment or around bridge piers depends on the sediment feeding conditions  
177 from upstream. Indeed, experiments can be designed with a certain rate of sediment supply from  
178 upstream (as generally expected in the field during flood events) or without coarse sediment  
179 supply (i.e., clear water conditions). Overall, the scour depth abutment has been considered as a  
180 function of sediment size, flow parameters, and the geometrical characteristics of the structure  
181 (Bonakdari et al., 2020; Firat and Gungor, 2009; Raudkivi and Ettema, 1983; Sheppard et al.,  
182 2004). This can be written as follows:

$$183 \quad d_s = f(U, U_c, l, b, g, K_s, d_{50}, h, \rho, \rho_s, \nu) \quad (1)$$

184 Where  $U$  = average flow velocity;  $U_c$  = critical sediment velocity;  $l$  = transverse abutment  
185 length;  $b$  = stream wise length;  $g$  = acceleration due to gravity;  $K_s$  = abutment form factor;  $d_{50}$  =  
186 median sediment diameter;  $h$  = approach flow depth;  $\rho$  = fluid density;  $\rho_s$  = sediment density;



187 and  $\nu$  = fluid kinematic viscosity. As  $K_s$  is the same for each cross section, and  $\rho, \rho_s$  are  
188 constant, these three parameters can be removed from the list. According to several approaches  
189 in literature, and considering that dimensionless parameters are generally to be preferred, it can  
190 be said that:

$$191 \quad \frac{d_s}{l} = f\left(F_e, \frac{d_{50}}{l}, \frac{d_{50}}{h}, \frac{h}{l}\right) \quad (2)$$

192 where  $F_e$  is defined as an excess abutment Froude number:

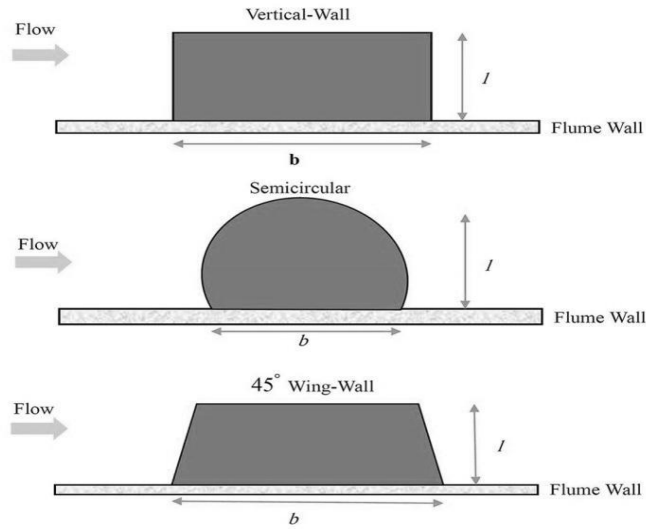
$$193 \quad F_e = U / \sqrt{gl(S-1)} \quad (3)$$

194 where  $S$  is the specific gravity of sediment defined as the ratio of sediment density to fluid  
195 density. Because  $b/l$  was constant for the dataset used in this paper (see next chapter) it was  
196 removed from the list of effective input parameters.

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## 198 ***2.2. Dataset collection and preparation***

199 In this study we used the Dey and Barbhuiya (2005) dataset referring to relative scour depth  
200 around bridge abutment ( $ds/l$ ) for clear water conditions using uniform bed sediments. The  
201 dataset of Dey and Barbhuiya (2005) consists of 295 runs carried out in the hydraulic laboratory  
202 of the technology institute in India for three abutment shape (i.e. each set is about 99 data). Their  
203 experiments were performed on a 20 m long, 0.9 m wide, and 0.7 m deep flume. They used three  
204 type of abutment shape, vertical wall, 45° wing wall, and semicircular which were made from  
205 Plexiglas in five different sizes (Fig 1 and Table 1 and 2).



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207 Fig 1. Schematic diagram of scouring at an abutment: (a) vertical wall, (b) semicircular and (c) 45° wing  
 208 wall. [ $b$  is the streamwise length and  $l$  is the transverse length of abutments]

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Each abutment was placed in a bed sediment recess of 2.4 m long, 0.9 m wide, and 0.3 m deep appended to the flume wall. A V-notch weir embedded at the inlet of the flume was used for measuring discharge. A mechanical point gauge was used for flow adjusting and finally measured through Vernier point gauge with a veracity of  $\pm 0.1$  mm. All experiments were carried out without feeding sediments (i.e., clear water) with  $U/U_c < 0.95$  for a period of 48-50 h, until equilibrium conditions (i.e., no further morphological changes) were reached. All experiments were performed in a condition of short abutment physical model (i.e.,  $h/l > 1$ ). Barbhuiya (2003) and Dey and Barbhuiya (2005) provide further details on the experimental runs and conditions. In the present work, after compiling the dataset, the data was spilt into two main groups randomly, with 70% of data (69 dataset) being used for developing the model, while the remaining 30% (30 dataset) was used for validating purpose.

221 Table 1. Main characteristics of the dimensionless data for the training (a) and the testing (b) dataset from  
 222 Dey and Barbhuiya (2005)

223

(a) Parameters	Vertical					Semi-circular					45 degree				
	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew
$d_{50}/l$	0.05	0.00	0.01	0.01	0.05	0.08	0.00	0.01	0.01	2.32	0.08	0.00	0.01	0.01	2.32
$h/l$	6.25	0.48	2.37	1.42	6.25	6.25	0.38	2.41	1.44	1.02	6.25	0.59	2.45	1.39	1.02
$d_{50}/h$	0.02	0.00	0.01	0.01	0.02	0.02	0.00	0.01	0.01	1.04	0.02	0.00	0.01	0.01	1.14
$Fe$	0.68	0.16	0.33	0.13	0.68	0.83	0.17	0.33	0.14	1.42	0.83	0.17	0.33	0.13	1.51
$ds/l$	3.70	0.98	2.07	0.64	3.70	3.38	0.71	1.67	0.62	0.73	3.78	0.76	1.76	0.63	1.02

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(b) Parameters	Vertical					Semi-circular					45 degree				
	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew
$d_{50}/l$	0.08	0.00	0.01	0.02	2.84	0.05	0.00	0.01	0.01	1.52	0.05	0.00	0.01	0.01	1.37
$h/l$	6.25	0.58	2.42	1.39	1.06	6.25	0.45	2.29	1.41	1.07	6.25	0.61	2.43	1.38	1.12
$d_{50}/h$	0.02	0.00	0.01	0.00	1.44	0.02	0.00	0.01	0.01	1.28	0.02	0.00	0.01	0.00	1.16
$Fe$	0.83	0.17	0.32	0.14	1.90	0.65	0.15	0.31	0.13	1.17	0.65	0.19	0.33	0.12	1.01
$ds/l$	4.35	1.13	2.08	0.71	1.37	3.10	0.65	1.58	0.60	0.86	3.15	1.00	1.75	0.62	0.77

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226 Table 2. Main characteristics of the dataset for the training (a) and the testing (b) dataset from Dey and  
 227 Barbhuiya (2005)

(a) Parameters	Vertical					Semi-circular					45 degree				
	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew
$d_{50}$ (m)	0.003	0.000	0.001	0.001	1.285	0.003	0.000	0.001	0.001	1.239	0.003	0.000	0.001	0.001	1.322
$h$ (m)	0.250	0.058	0.161	0.065	-0.169	0.250	0.050	0.164	0.064	-0.330	0.250	0.059	0.163	0.065	-0.190
$ds$ (m)	0.293	0.068	0.156	0.055	0.715	0.258	0.055	0.123	0.056	1.022	0.274	0.053	0.125	0.052	0.983
$l$ (m)	0.120	0.040	0.080	0.028	0.000	0.130	0.040	0.080	0.032	0.201	0.100	0.040	0.075	0.024	-0.361
$b$ (m)	0.240	0.080	0.160	0.057	0.000	0.260	0.080	0.080	0.063	0.201	0.300	0.120	0.075	0.071	-0.361

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(b) Parameters	Vertical					Semi-circular					45 degree				
	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew	Max	Min	Mean	STD	Skew
$d_{50}$ (m)	0.003	0.000	0.001	0.001	1.572	0.002	0.000	0.001	0.001	1.109	0.003	0.000	0.001	0.001	1.347
$h$ (m)	0.250	0.058	0.166	0.063	-0.257	0.250	0.050	0.158	0.069	-0.076	0.250	0.059	0.163	0.063	-0.324
$ds$ (m)	0.287	0.078	0.153	0.051	0.840	0.240	0.058	0.112	0.045	1.172	0.269	0.061	0.127	0.053	1.174
$l$ (m)	0.120	0.040	0.079	0.029	-0.008	0.130	0.040	0.078	0.034	0.633	0.100	0.040	0.075	0.023	-0.352
$b$ (m)	0.240	0.080	0.159	0.059	-0.008	0.260	0.080	0.078	0.068	0.633	0.300	0.120	0.075	0.070	-0.352

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### 231 2.3. Input combination

232 Because some input parameters are more important or relevant than others, it is important to  
 233 exclude those that hamper the modeling performance without improving the effectiveness of the  
 234 modeling. To select the best input array, four different input combinations were constructed  
 235 based on the Pearson correlation coefficient and finally tested in order to find the most effective  
 236 one. To start with, the parameter with the highest degree of Pearson correlation coefficient ( $r$ )

237 was considered as the first input parameter to the model. The assumption is that the parameter  
 238 with the highest correlation with the output has a better ability to predict the output with higher  
 239 accuracy. Then, the next parameter with the next highest r value was added to the first input and  
 240 the selection “input No.2” was defined. This process continued until the parameter with the  
 241 lowest r value was added to the combination of input parameters and the selection “input No.4”  
 242 was defined (Table 3). The most effective input combination was identified by comparing the  
 243 effectiveness of each input combination using the root mean square error (RMSE).

244 Table 3. Selection of the different input parameters.

Input No.	Input parameters	Outputs	Input parameters	Outputs
1	$F_e$	$ds/l$ ( <i>V, S</i> )	$F_e$	$ds/l$ ( $45^\circ$ )
2	$F_e, d_{50}/l$	$ds/l$ ( <i>V, S</i> )	$F_e, d_{50}/l$	$ds/l$ ( $45^\circ$ )
3	$F_e, d_{50}/l, h/l$	$ds/l$ ( <i>V, S</i> )	$F_e, d_{50}/l, d_{50}/h$	$ds/l$ ( $45^\circ$ )
4	$F_e, d_{50}/l, h/l, d_{50}/h$	$ds/l$ ( <i>V, S</i> )	$F_e, d_{50}/l, d_{50}/h, h/l$	$ds/l$ ( $45^\circ$ )

*V, S* and  $45^\circ$  are Vertical – Wall, Semicircular and  $45^\circ$ Wing – Wall

246  
 247 **2.4. Sensitivity analysis**

248 A sensitivity analysis was performed in order to determine the effectiveness of each input  
 249 parameter. To do that, at first all parameters were combined and considered as an input to the  
 250 model (i.e. No. Total). Next, four different combinations of parameters were considered (i.e. No.  
 251 A, B, C, and D) and in each combined parameter one of the input parameters was removed  
 252 (Table 4). Similar to the previous section, RMSE was used as criterion to determine the range of  
 253 changes, as removing the most effective parameter would lead to the highest degree of error.

254 Table 4. Parameters considered in the sensitivity analysis

No.	Input parameters	Removed parameter
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A	$Fe, d_{50}/l, h/l$	$d_{50}/h$
B	$Fe, d_{50}/l, d_{50}/h$	$h/l$
C	$Fe, h/l, d_{50}/h$	$d_{50}/l$
D	$d_{50}/l, h/l, d_{50}/h$	$Fe$
Total	$Fe, d_{50}/l, h/l, d_{50}/h$	---

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256 **2.5. Models parameter optimization**

257 Apart from the quality of the dataset, a proper selection of input parameters and the modeling  
 258 prediction capability determine the optimum values for each model operators and have a  
 259 significant effect on the predictive power of the model. In the present study, a trial-and-error  
 260 approach was applied to determine the optimum value for each operator. The models were first  
 261 run using a range of default values, and then the values were adjusted arbitrary until converging  
 262 to optimal values which led to the lowest RMSE.

263

264 **2.6. Model theory background**

265 **2.6.1. Kstar**

266 This is an instance-base model that classifies an instance by contrasting it to a pre-classified  
 267 sample dataset. Similar examples lead to similar classifications and this is the main assumption  
 268 of the Kstar model. The associated elements of an instance-based trainee are the distance  
 269 function that defines how similar two examples are, and the classification function that  
 270 determines how similar examples give the new example an ultimate classification (Cleary and  
 271 Trigg, 1995). The K-star method employs entropic measurements related to the probability of  
 272 turning a sample into another by selecting randomly between all feasible transformations. The  
 273 transformation of an example into another one is attained by mapping one instance to another by

274 determining a finite set of transformation, and then one instance ( $m$ ) is converted into a finite  
 275 sequence of transformations ( $n$ ) beginning at (a) and ending at (b). Assuming the defined  
 276 transformation  $S$  and a value of this set  $s$ , maps  $I \rightarrow I$ . For mapping instance with itself  $\beta$  is  
 277 employed in  $S(\beta(a) = a)$ . The parameter  $\beta$  terminates the set of all codes of  $S^*$  as  $P$ .

$$278 \quad \bar{s}_a = s_n(s_{n-1}(\dots s_1(a) \dots)) \quad s = s_1 \dots s_n \quad (4)$$

279 The probability function of  $S^*$  is  $P$  which satisfies properties as:

$$280 \quad 0 \leq \frac{p(\bar{s}u)}{p(\bar{s})} \leq 1 \quad (5)$$

$$281 \quad \sum_u p(\bar{s}u) = p(\bar{s}) \quad (6)$$

$$282 \quad p(\Lambda) = 1 \quad (7)$$

$$283 \quad \sum_{s \in p} p(\bar{s}) = 1 \quad (8)$$

284 The probability function  $P^*$  is defined as:

$$285 \quad P^*\left(\frac{b}{a}\right) = \sum_{s \in p: s(a)=b} P(s) \quad (9)$$

286 Finally, the Kstar function defined as:

$$287 \quad k^*\left(\frac{b}{a}\right) = -\log_2 P^*\left(\frac{b}{a}\right) \quad (10)$$

### 288 **2.6.2. Bagging model**

289 Machine learning implementations suggest that any given learning model can outperform all  
 290 others for a specific issue or for a specific subset of input data, but it is rare to find a single  
 291 expert that achieves good outcomes on an entire given problem (Dietterich, 2000). Bagging is

292 one of the most well-known ensembles learning algorithms (Oza, 2005), and is also known as  
293 bootstrap aggregation (Breiman, 1996). The bagging model can improve the classification  
294 precision in machine learning (ML) and by decreasing the variance can leadsto prevent  
295 overfitting. This model is commonly applied to decision tree-based algorithms (Dietterich,  
296 2000). The bagging model procedure follows three steps: a) bootstrap samples are collected by  
297 resampling as arbitrarily of the training dataset to develop a set of training subsets; b) several  
298 classifier-based models are designed using each of the sub-sets; and finally, c) the terminal  
299 method comprises of the aggregation of all classifier-based models.

### 300 ***2.6.3. Dagging model***

301 The Dagging algorithm presented by Ting and Witten (1997) is an ensemble of algorithms. Like  
302 the bagging, the approach focuses at achieving a reliable model of classification by integrating  
303 the poor learners trained on various samples of the training set. However, the Dagging model  
304 utilizes disjointed, stratified samples instead of bootstrapping, and it is a powerful method when  
305 the individual classifiers have a bad time complexity.

### 306 ***2.6.4. Random Subspace model***

307 The Random Subspace method (RSS; Ho, 1998) is another ensemble learning technique that  
308 combines several classifiers trained on randomly chosen subspace features. This algorithm is  
309 randomly selected by the original training set to make the training subset (Kotsiantis, 2011).  
310 Therefore, the series features a subset of each sub-classifier training at the final prediction  
311 outcomes achieved by a combination of voting methods (Sun and Zhang, 2007). When  
312 considering the situation with  $n$  observations in a  $k$  dimensional space, then:

313  $\left\{ \left( (x_{1j}, x_{2j}, \dots, x_{ij}, \dots, x_{kj}) \middle| x_{ij} \right) \right\}, \text{ where } i \in \{1, n\}, j \in \{1, k\}$  (11)

314 where  $i$  and  $j$  are the number of observation and variables, respectively. The RSS developed by  
 315 selecting the variables randomly is shown as follows:

316  $\left\{ \left( (x_{1j}, x_{2j}, \dots, x_{ij}, \dots, x_{kj}) \middle| x_{ij} \right) \right\}, \text{ as } \begin{cases} x_{ij} = x_{ij} & \text{for } i \in I \\ x_{ij} = \text{Null} & \text{for } i \notin I \end{cases}$  (12)

317 in which  $I$  is  $k'$  dimensional subsect and  $k' \leq k$ . Every time, a random producer from 1 to  $k'$  is  
 318 employed to choose a variable to be applied in the subspace, and the procedure is repeated  $k'$   
 319 times. As an outcome,  $N$  random subspaces are generated as various filters for the identified  
 320 issue. On top of those filters, a similar algorithm is applied to produce various decision agents.  
 321 This method represents a type of stochastic discrimination to improve prediction performance by  
 322 combining poor models with no maximum discriminating power for the same issue (Ho, 1998).

323 **2.6.5. Random Committee model**

324 The Random Committee (RC) approach produces a set of main classifiers (random trees) and  
 325 creates their estimation by combining predictions of probability. Every main classifier is based  
 326 on similar data but employs another gene random number. It only becomes meaningful when  
 327 randomizing the main classifier, otherwise all classifiers will be equivalent.

328 **2.6.6. Weighted Instance Handler Wrapper model**

329 In this algorithm, a general wrapper technique around any classifier allows for weighted  
 330 instances support. This approach benefit from a weight resampling if the interface is not enforced  
 331 by the base classifier and instances weights other than 1.0. The learning data is transferred to the



332 base classifier by default when it can manage instance weights. Anyway, implementing  
333 resampling technique with weights is applicable as well.

### 334 **2.6.7. Dey and Barbhuiya equation**

335 In their original paper, Dey and Barbhuiya (2005) proposed empirically derived equations for  
336 each shape of abutments as such:

$$337 \quad \frac{d_s}{l} = 7.281(F_e)^{0.314} \left(\frac{h}{l}\right)^{0.128} \left(\frac{l}{d_{50}}\right)^{-0.167} \quad \text{for Vertical wall abutments} \quad (13)$$

$$338 \quad \frac{d_s}{l} = 8.319(F_e)^{0.312} \left(\frac{h}{l}\right)^{0.101} \left(\frac{l}{d_{50}}\right)^{-0.213} \quad \text{for } 45^\circ \text{ wall abutments} \quad (14)$$

$$339 \quad \frac{d_s}{l} = 8.689(F_e)^{0.192} \left(\frac{h}{l}\right)^{0.103} \left(\frac{l}{d_{50}}\right)^{-0.296} \quad \text{for Semicircular abutments} \quad (15)$$

340 where  $d_{50}$  is the median diameter of sediment particles,  $d_s$  is the equilibrium scour depth,  $h$  is the  
341 approaching flow depth,  $l$  is the transverse length of abutment, and  $F_e$  is the excess abutment  
342 Froude number

### 343 **2.6.8. Muzzammil equation**

344 The performance of three ANFIS, ANN and conventional regression-based models for  
345 estimating scour depth in clear-water condition was investigated by Muzzammil (2010). He  
346 obtained a conventional regression model which depended on four different nondimensional  
347 parameters to calculate the scour depth at the abutments as:

$$348 \quad \frac{d_s}{l} = 9.694K_s(F_e)^{0.648} \left(\frac{h}{l}\right)^{0.04} \left(\frac{d_{50}}{l}\right)^{-0.075} \quad (16)$$

349 Equation (16) was proposed for all three different geometry of abutments (vertical-wall, 45°  
350 wing-wall, and semicircular).

### 351 **2.7. Model evaluation**

352 All approaches are finally compared in terms of their power of prediction. Here a portion (30%)  
 353 of the original dataset was used for validation purposes, and we used both graphical methods  
 354 (line graphs, scatter plots and violin plots) and quantitative metrics to evaluate the performance  
 355 of each approach. A ranking of performance was achieved using quantitative metrics including  
 356 RMSE, relative RMSE (RRMSE), Nash-Sutcliffe efficiency index (NSE), Willmott's index of  
 357 agreement (WI), and Legates and McCabe coefficient of efficiency (LM) that were computed as  
 358 follows:

$$359 \quad RMSE = \sqrt{\frac{1}{N} \sum_{i=1}^N (d_s^{Obs} - d_s^{Pre})^2}, \quad 0 \leq RMSE \leq +\infty \quad (17)$$

$$360 \quad RRMSE = \frac{\sqrt{\frac{1}{N} \sum_{i=1}^N (d_s^{Obs} - d_s^{Pre})^2}}{\sum_{i=1}^N d_s^{Obs}}, \quad 0 \leq RRMSE \leq +\infty \quad (18)$$

$$361 \quad NSE = 1 - \left[ \frac{\sum_{i=1}^N (d_s^{Obs} - d_s^{Pre})^2}{\sum_{i=1}^N (d_s^{Obs} - \bar{d}_s^{Obs})^2} \right], \quad -\infty < NSE \leq 1 \quad (19)$$

$$362 \quad WI = 1 - \left[ \frac{\sum_{i=1}^N (d_s^{Obs} - d_s^{Pre})^2}{\sum_{i=1}^N (|d_s^{Pre} - \bar{d}_s^{Obs}| + |d_s^{Obs} - \bar{d}_s^{Obs}|)^2} \right], \quad 0 \leq WI_E \leq 1 \quad (20)$$

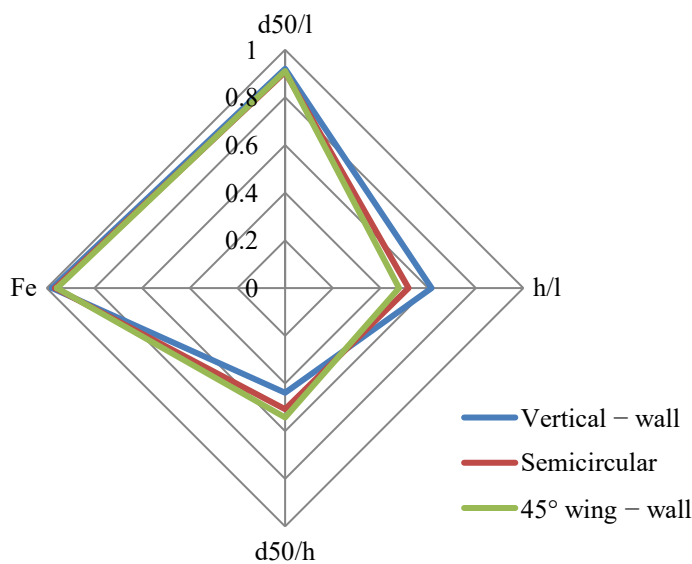
$$363 \quad LM = 1 - \left[ \frac{\sum_{i=1}^N |d_s^{Pre} - d_s^{Obs}|}{\sum_{i=1}^N |d_s^{Obs} - \bar{d}_s^{Obs}|} \right], \quad 0 \leq LM_E \leq 1 \quad (21)$$

364 where  $N$  is the number of data sample, while  $d_s^{Pre}$ ,  $d_s^{Obs}$ , and  $\bar{d}_s^{Obs}$ , are the predicted, observed  
 365 and mean of the observed scour depth, respectively.

366 **3. Results and discussion**

367 **3.1. The importance of the input variables**

368 Each input parameter has a different relative effectiveness on the result. The relative importance  
369 of these parameters for each abutment shape was assessed through the Pearson correlation  
370 coefficient ( $r$ ) and is shown in Fig 2. Results reveal that the  $F_e$  parameter has the highest effect  
371 on the scour depth prediction at each shape of Vertical – wall ( $r = 0.978$ ), Semicircular ( $r = 0.964$ )  
372 and  $45^\circ$  wing – wall ( $r = 0.957$ ), followed by  $d_{50}/l$  ( $r = 0.920, 0.906$  and  $0.910$  respectively),  $h/l$  ( $r$   
373  $= 0.614, 0.518$  and  $0.528$  respectively) and  $d_{50}/h$  ( $r = 0.439, 0.508$  and  $0.512$  respectively).



374

375 Fig 2. The relative importance of each input parameter using Pearson correlation coefficient

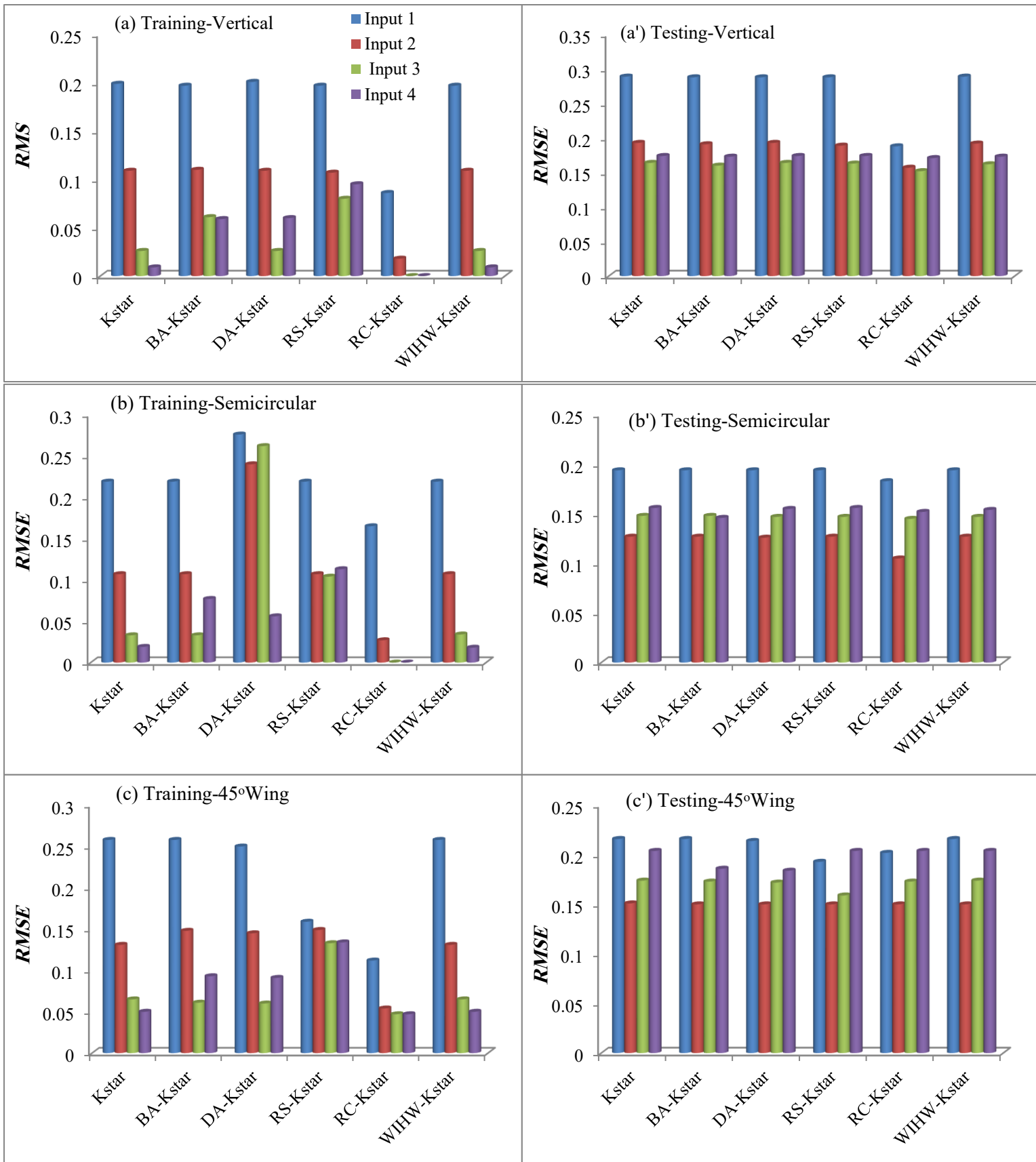
376 **3.2. Assessing the best input combination**

377 Although incorporating more input parameters in most of the cases can improve the performance  
378 of each algorithm in the training phase, including more parameters lead to more errors and to  
379 over-complicated algorithms (Fig 3a-c). For vertical-wall shape abutments, the best input  
380 combination is the No.3 (parameters  $F_e, d_{50}/l, h/l$ ), as involving the additional parameter  $d_{50}/h$

381 reduces the modeling prediction power and lead to more error. This result is in accordance with  
382 the outcomes of the correlation coefficient which showed that the  $d_{50}/h$  parameter has the lowest  
383 correlation with scour depth process.

384 For semicircular and  $45^\circ$  wing shape abutment, the input combination No.2 ( $F_e$  and  $d_{50}/l$ ) has the  
385 highest effectiveness if compared to other input parameter combinations and results show that  
386 for semicircular shape abutment incorporating  $h/l$  and  $d_{50}/h$  lead to more degree of error.  
387 Although  $h/l$  is included on the empirical equations of Dey and Barbhuiya (2005) and  
388 Muzzammil (2010), our results show that this parameter is not crucially important in all cases  
389 and lead to more error in the modeling process.

390 Results show that the best input combinations have a 9.04%, 16.93% and 28.57% higher  
391 performance for vertical, semicircular and  $45^\circ$  wing abutment shape respectively, if compared to  
392 the worst input combination. This proves that determining the best input combination is  
393 significantly effective on the result and one of the most important steps in modeling process.  
394 Also, with the same input combination the result for different algorithms would be different,  
395 which reflects the different computation structures of the algorithms that each models developed  
396 is based on. Bonaldari et al. (2020) revealed that the best input combination for the Extreme  
397 Learning Machine (ELM) model is one in which all parameters ( $F_e$ ,  $h/l$ ,  $d_{50}/l$ , and  $K_s$ ) are  
398 involved. Our results differ in this sense likely due to the different model structures and different  
399 computational capabilities.



400 Fig 3. Use of quantitative metrics (RMSE) to determine the best input combination for the

401 models (the dataset is split in training and testing components).

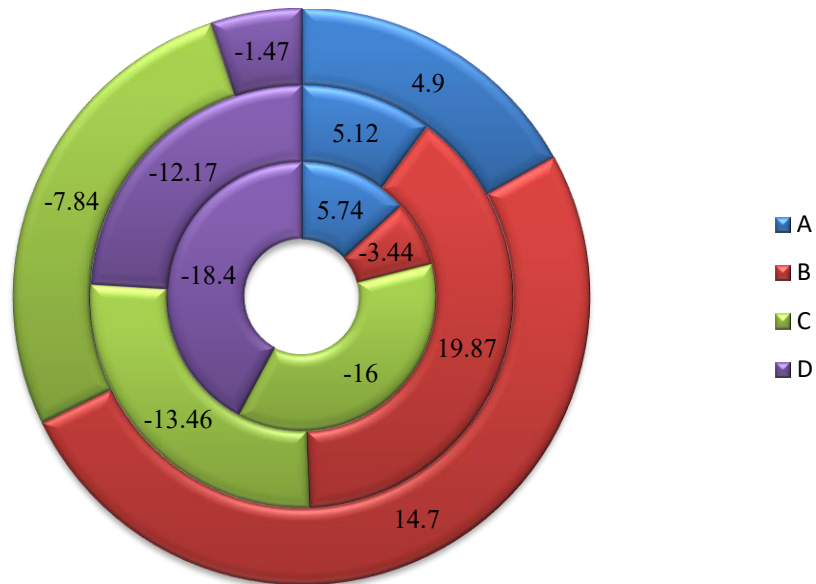
### 402 **3.3. Sensitivity analysis**

403 A sensitivity analysis was carried out to determine the extent to which each parameter is  
404 effective on the results (Fig 4 and Table 4). According to the results, for vertical abutment shape,  
405 removing  $d_{50}/h$ ,  $h/l$ ,  $d_{50}/l$  and  $F_e$  implies a 5.74%, -3.44%, -16% and -18.4% change in the result  
406 (in terms of RMSE) revealing that  $F_e$  has the most significant effect on the results followed by  
407  $d_{50}/l$ ,  $d_{50}/h$  and  $h/l$ . Also, removing  $h/l$ ,  $d_{50}/l$  and  $F_e$  increased RMSE while adding the  $d_{50}/h$  lead  
408 to a higher error. For semicircular shape abutment, removing  $d_{50}/h$ ,  $h/l$ ,  $d_{50}/l$  and  $F_e$  causes a  
409 5.12%, 19.87%, -13.46% and -12.17% change in the results and shows that  $h/l$  is the most  
410 effective parameter on scour depth prediction for semicircular shape abutment followed by  $d_{50}/l$ ,  
411  $F_e$  and  $d_{50}/h$ . However, incorporating  $d_{50}/l$  and  $F_e$  and removing  $d_{50}/h$ ,  $h/l$  lead to higher  
412 performances. Although removing  $h/l$  cause higher changes in the results, incorporating  $h/l$  and  
413  $d_{50}/h$  leads to more error and reduces the models prediction power. For the 45° wing shape,  
414 removing  $d_{50}/h$ ,  $h/l$ ,  $d_{50}/l$  and  $F_e$  causes a 4.9%, 14.7%, -7.84% and -1.47% changes on the scour  
415 depth prediction. Indeed,  $h/l$  is the most important parameter followed by  $d_{50}/l$ ,  $d_{50}/h$  and  $F_e$ ,  
416 respectively. Also, similar to semicircular shape abutment, incorporating  $d_{50}/l$  and  $F_e$  and  
417 removing  $d_{50}/h$ ,  $h/l$  leads to higher performances. This is in accordance with the outcomes of the  
418  $r$  value and the determination of the most effective input parameter combination. Also, our result  
419 is in accordance with Bonakdari et al. (2020) which stated that the relative median sediment  
420 diameter ( $d_{50}/l$ ) is the most effective parameter for the prediction of scour depth. Bonakdari et al.  
421 (2020) also stated that  $h/l$  is effective on the result and removing this parameter cause to about  
422 1% higher error (which is overall not very significant). The opposite result can be obtained from  
423 different models structure and combining all dataset (rather than splitting them based on the

424 abutment shape as in the present study). Our results are in accordance with the finding of  
425 Mohammadpour et al. (2013) that showed that incorporating  $h/l$  can lead to higher errors in the  
426 determination of the scour depths.

427 According to our results, the abutment geometry (i.e.,  $l$ ) has a significant effect on the scour  
428 occurrence process. As  $45^\circ$  and semicircular shape are more similar to each other if compared to  
429 the triangular shape, the effective parameters for  $45^\circ$  and semicircular shape are the same and  
430 different from those selected for the triangular shape.

431 Our results show that the approach flow depth ( $h$ ) is not very effective on the overall scour depth,  
432 while flow velocity, transverse abutment length, and median sediment diameter are important.  
433 Azimi et al., (2019), Moradi et al., (2019) and Bonakdari et al., (2020) revealed that their best  
434 input combination was a combination of  $F_e$ ,  $h/l$ ,  $K_s$ ,  $d_{50}/l$ . This definitely shows that the structure  
435 of the modeling approach has a crucial role on the selection of the input combination, which is  
436 resulting from different structure of each model. In our study, the best input combination for  $45^\circ$   
437 wing wall and semicircular shapes involves  $F_e$  and  $d_{50}/l$  while for vertical wall the best  
438 combination includes  $F_e$ ,  $d_{50}/l$  and  $h/l$ .



439

440 Fig 4. Percentage of changes applied for sensitivity analysis (the inner, middle and outer circles  
 441 represent the vertical, semicircular, and 45° wing abutment shapes, respectively).

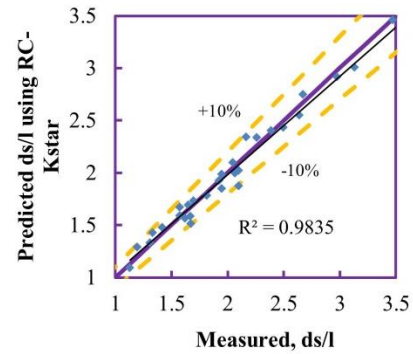
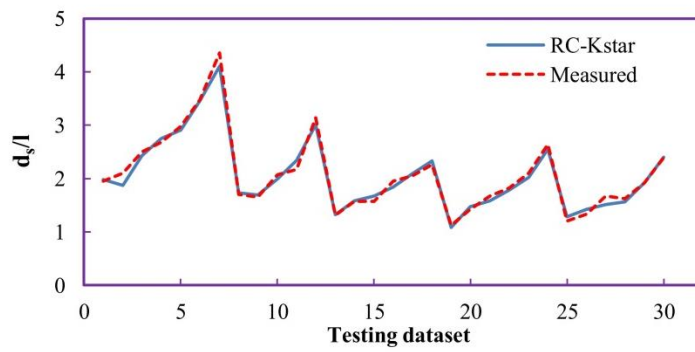
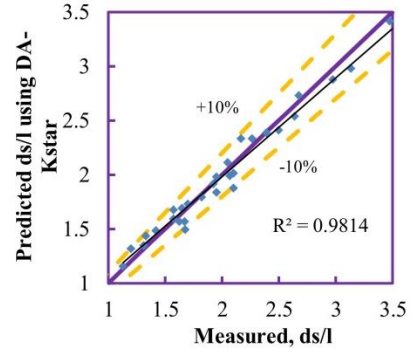
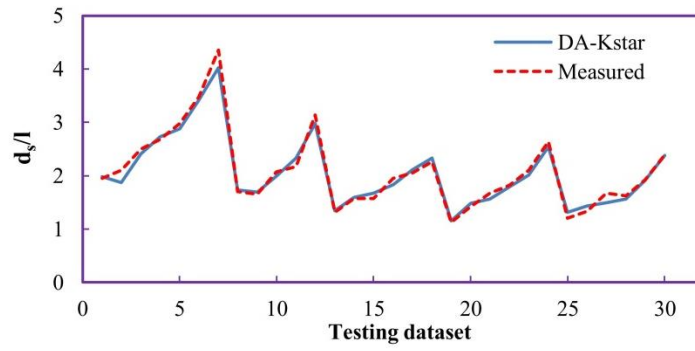
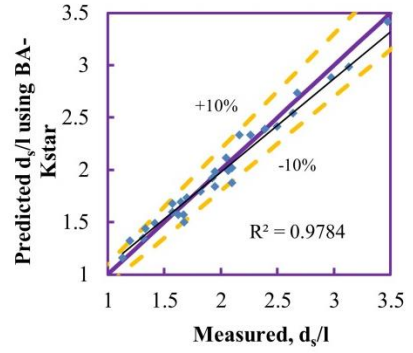
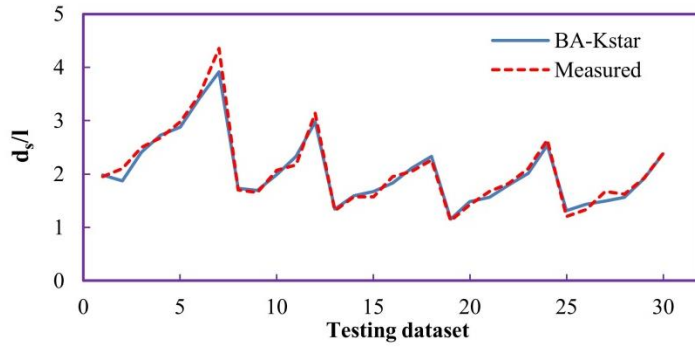
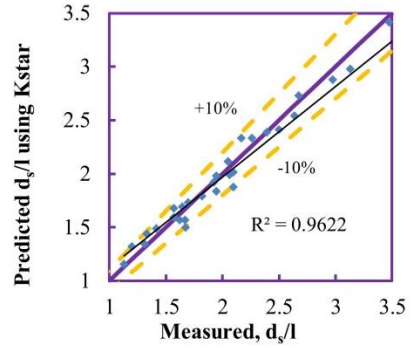
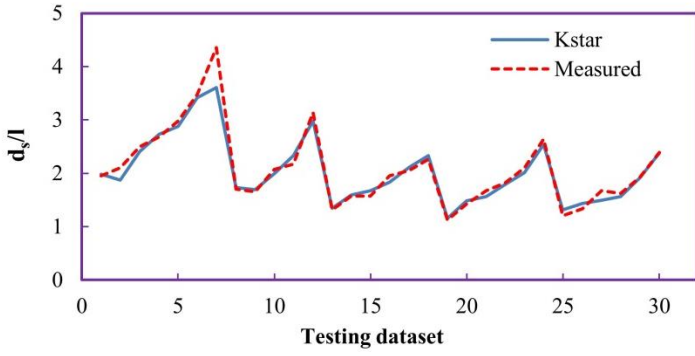
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443 **3.4. Evaluation of models performances**

444 A visual comparison of the prediction power of the different machine learning models in terms  
 445 of line graph and scatter plot is presented in Figure 5, 6 and 7 for vertical, semicircular and 45°  
 446 wing, respectively. The figures present the values of scour depth obtained from the experimental  
 447 data, and the values predicted by the stand-alone models of Kstar and its hybrid models of BA-  
 448 Kstar, DA-Kstar, RC-Kstar, and WIHW-Kstar. Figure 6 also presents the results obtained using  
 449 the empirical equations developed by Dey and Barbhuiya (2005) and Muzzammil (2010), and  
 450 shows how all hybridized algorithm could enhance the performance of standalone Kstar  
 451 algorithms while the empirical equations appear substantially overperformed.

452



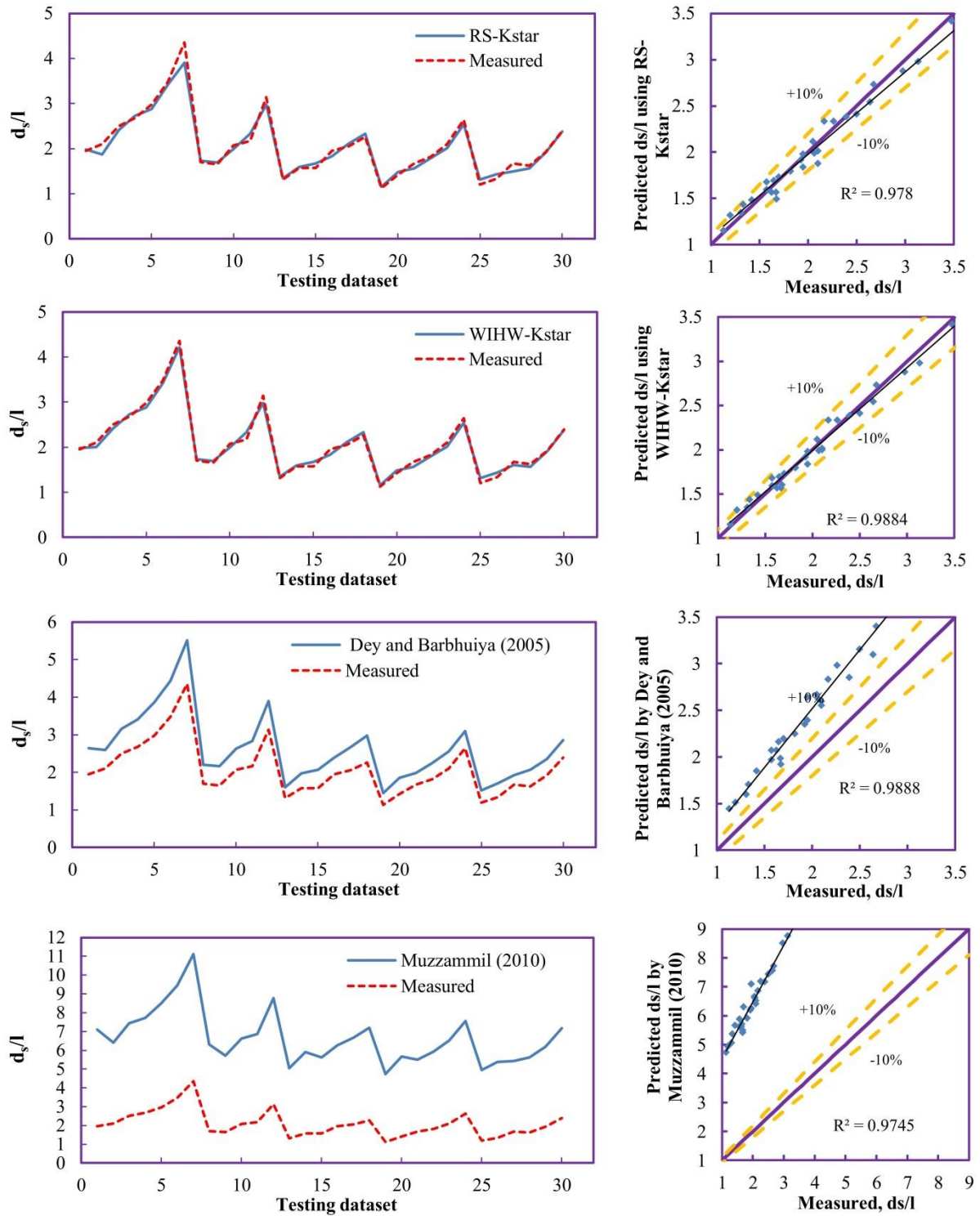


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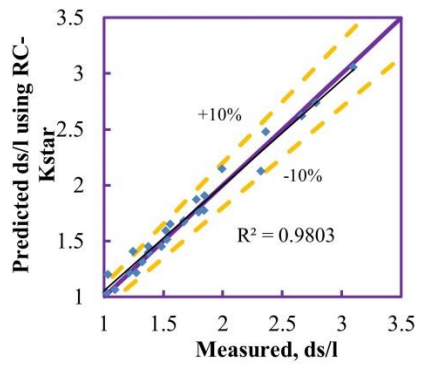
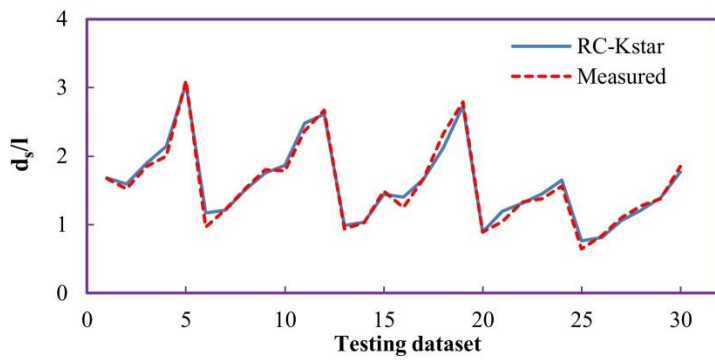
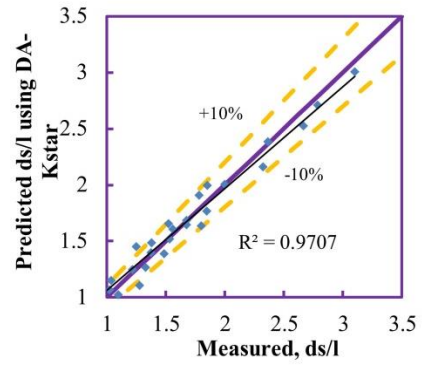
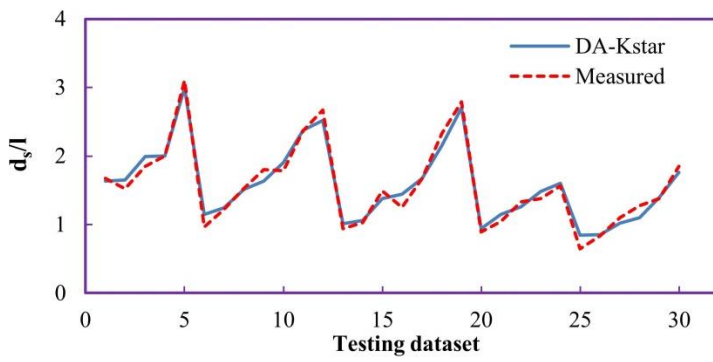
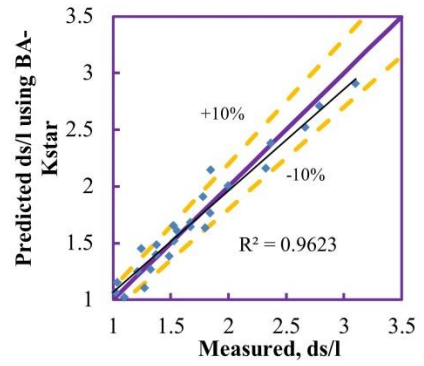
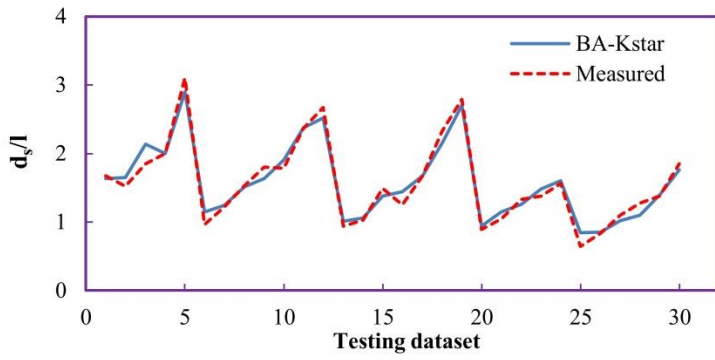
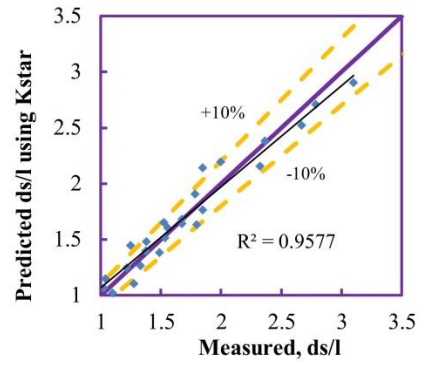
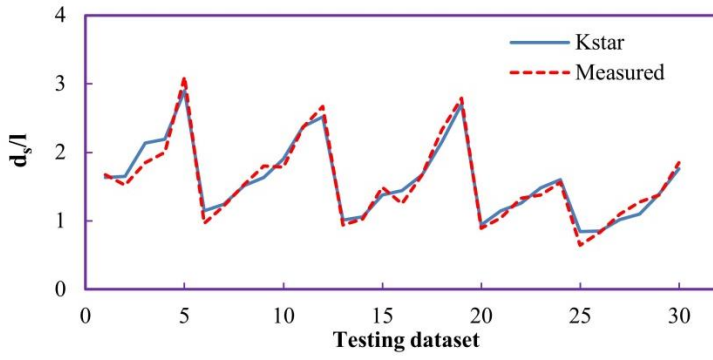
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458 Fig 5. Line-graph and scatter plot of predicted vs. measured  $d_s/l$  for vertical- wall shape abutment

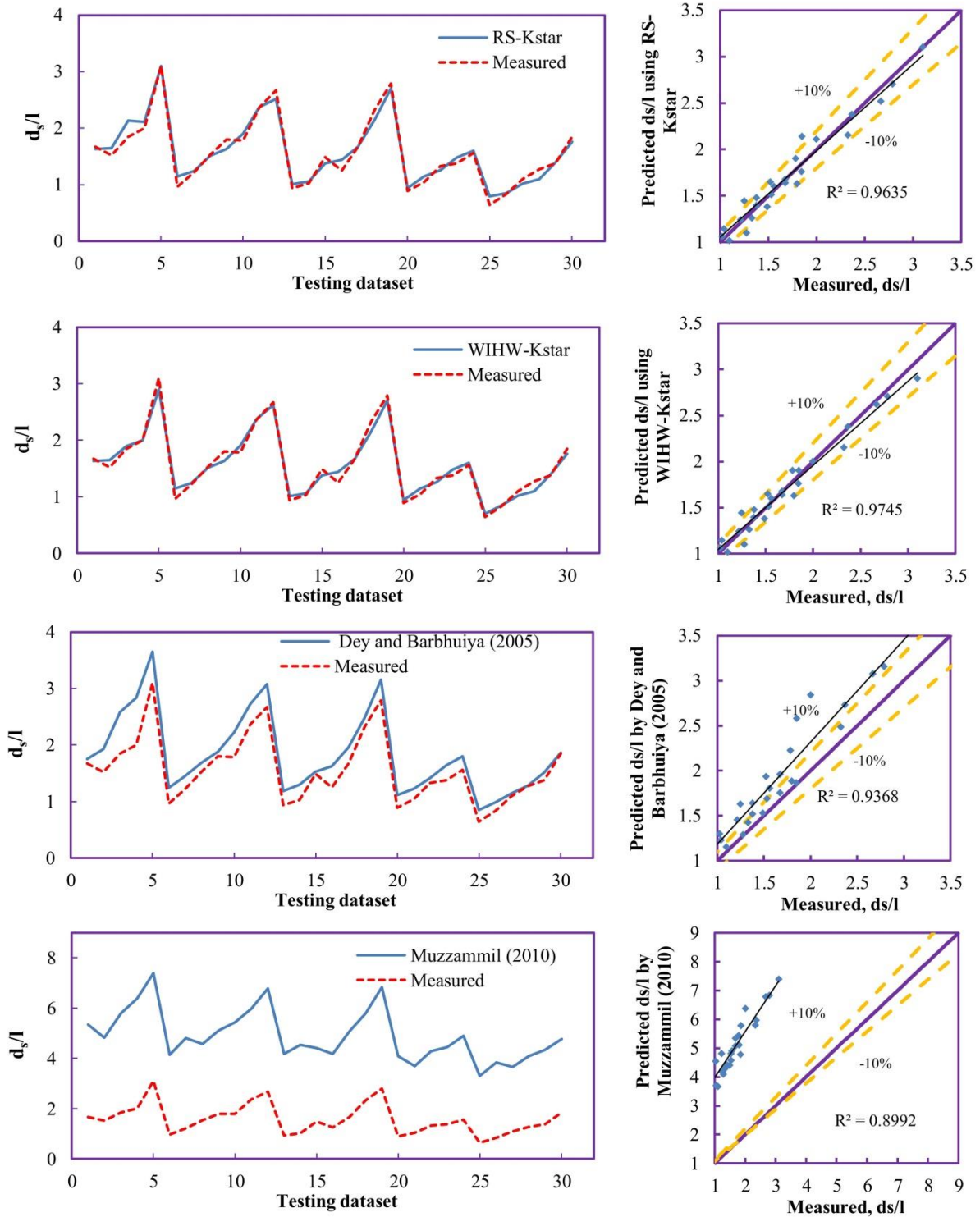
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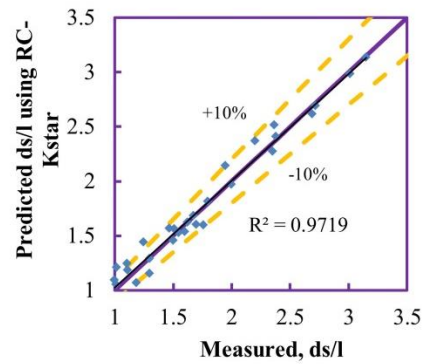
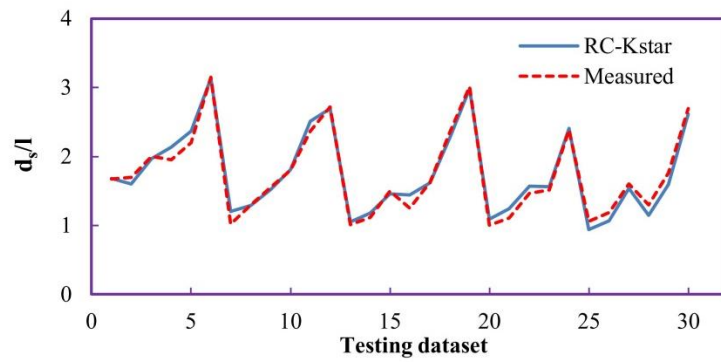
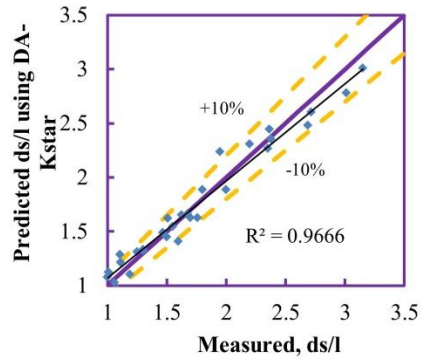
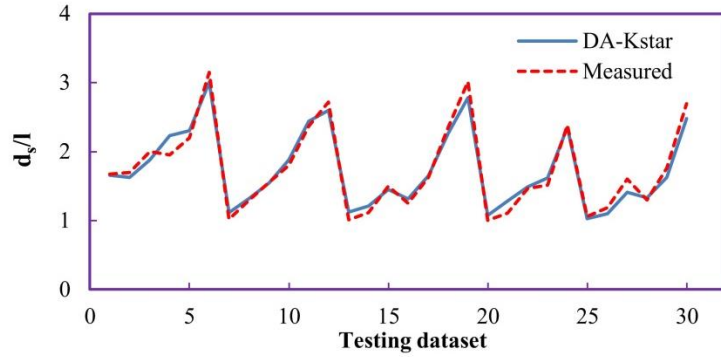
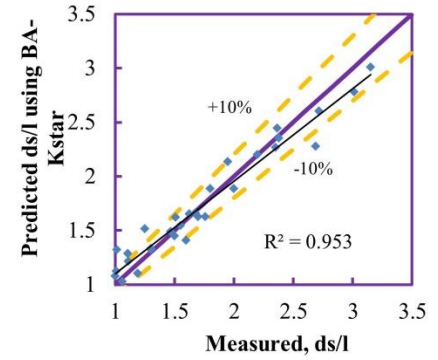
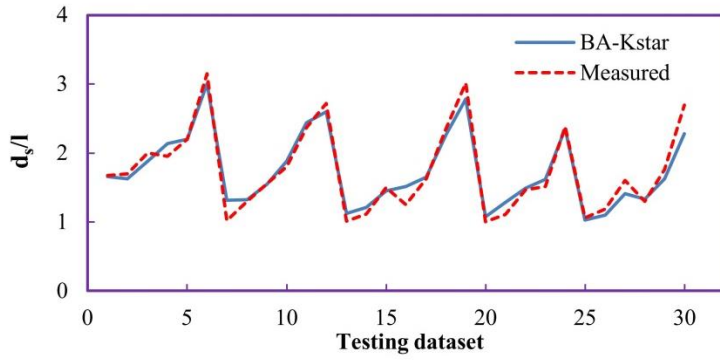
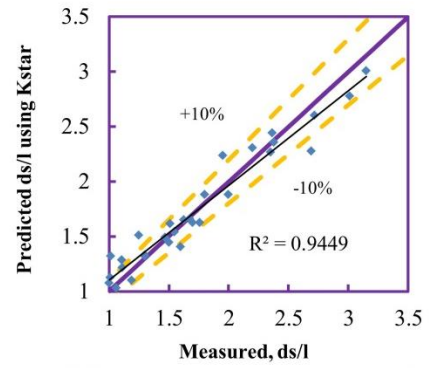
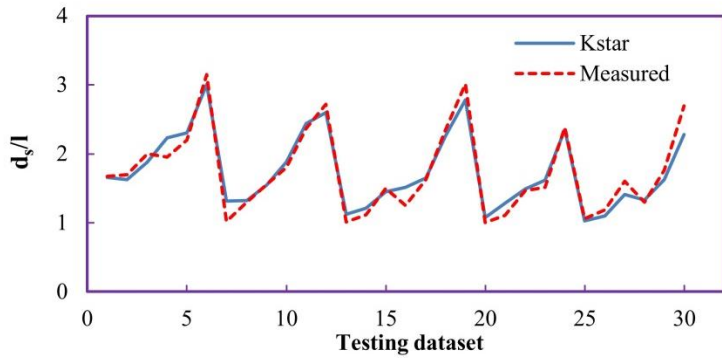
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464 Fig 6. Line-graph and scatter plot of predicted vs. measured  $ds/l$  for semicircular shape abutment

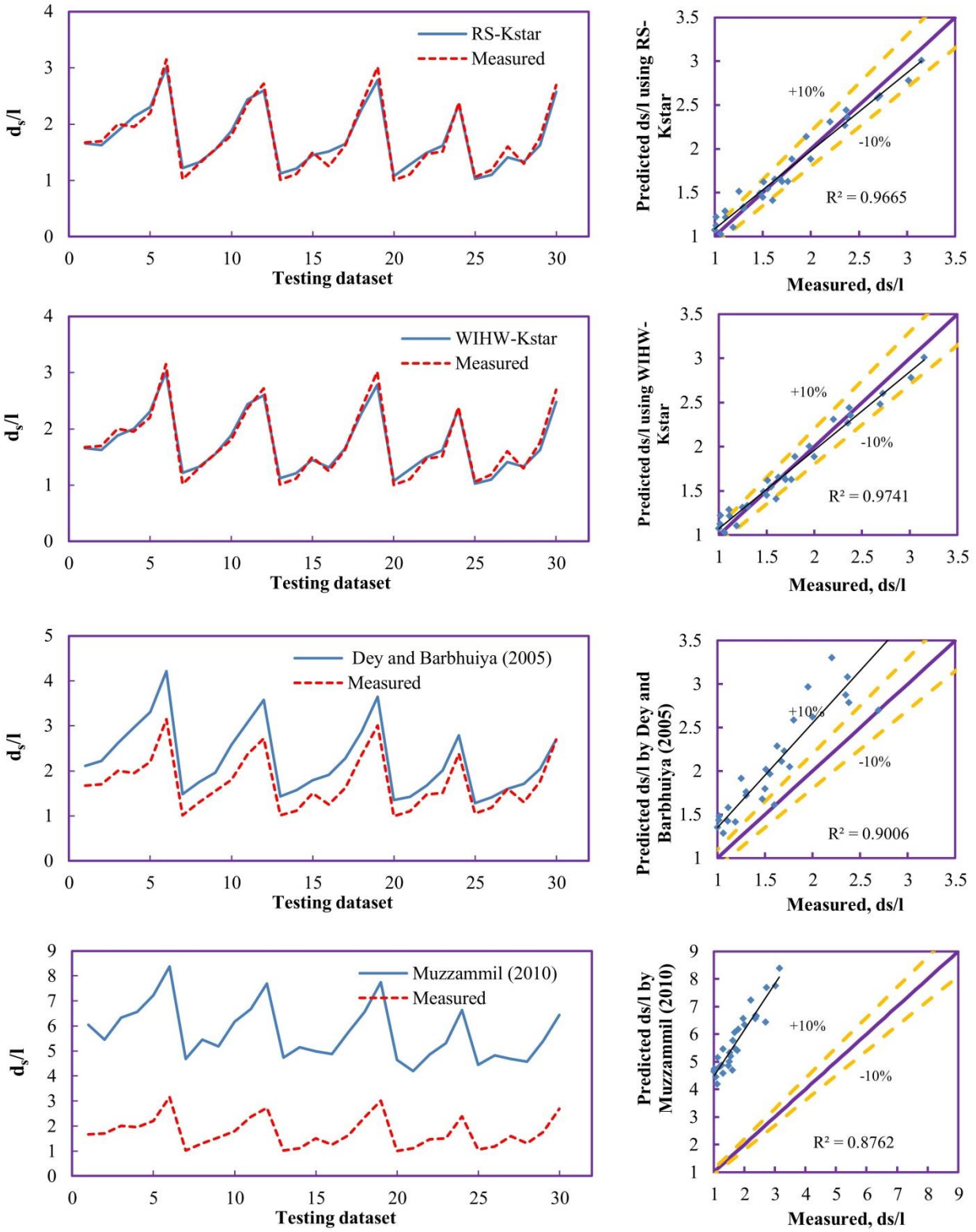
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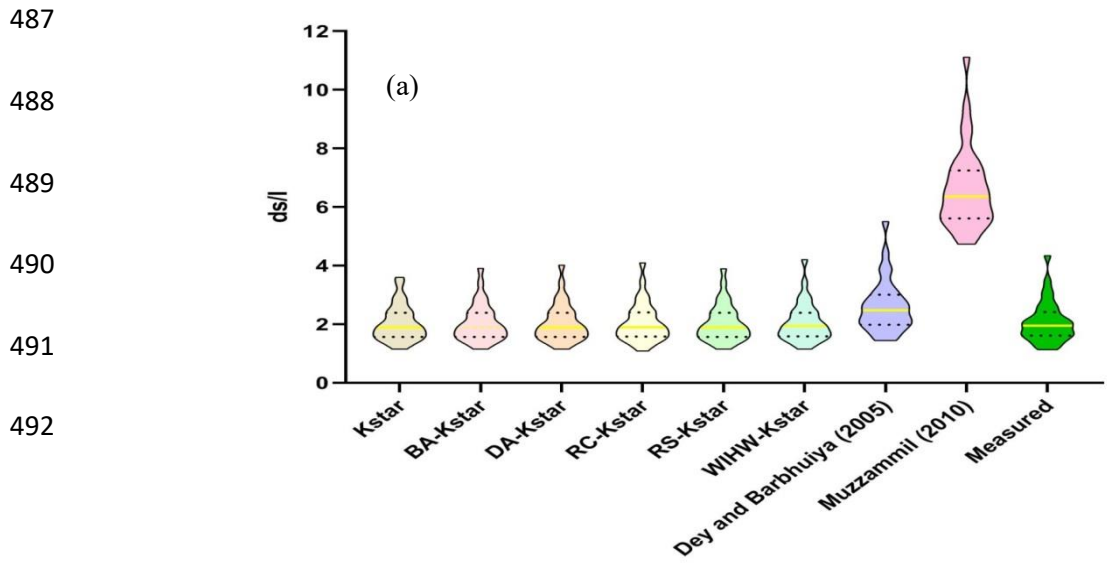
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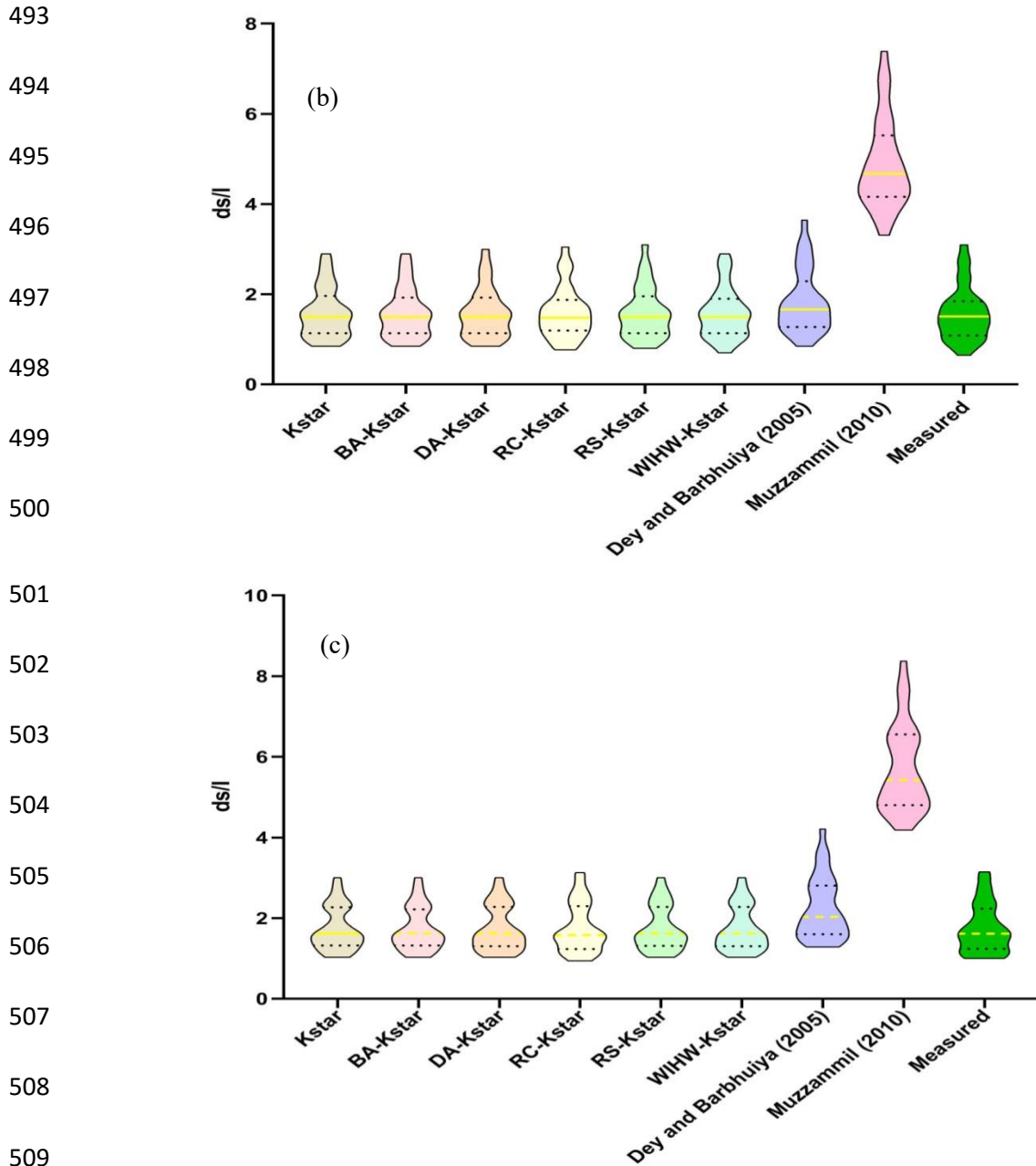


469

470 Fig 7. Line-graph and scatter plot of predicted vs. measured  $d_s/l$  for  $45^\circ$  wing shape abutment

471 The violin plots of Figure 8 show that the WIHW-Kstar is able to predict the maximum  $ds/l$   
 472 values more accurately than other algorithms for vertical abutment. All developed algorithms are  
 473 able to predict the first and third quartile and median relative scour depth values accurately while  
 474 RC-Kstar predicted the minimum scour depth close to the measured values. Results also show  
 475 that the equation proposed by Dey and Barbhuiya (2005) is more accurate than the equation  
 476 proposed by Muzzammil (2010). Overall, the shape of violin plot of WIHW-Kstar is more  
 477 similar to measured values compare to other models which have a similar distribution of data  
 478 with measured dataset (Fig 8a). Although the violin plot generated by Dey and Barbhuiya (2005)  
 479 equation have a similar shape of the measured values, their empirical equation fails to predict  
 480 maximum, minimum, and median values with accuracy. Overall, the RC-Kstar model has a more  
 481 similar shape of violin plot if compared with the measured values (Fig 8b). The RS-Kstar  
 482 predicted better the maximum values while the WIHW-Kstar predicted better the minimum  
 483 values. For  $45^\circ$  wing wall, all violin plot shapes of predicted dataset are far from the measured  
 484 data. The RC-Kstar is able to predict the maximum values accurately while RS-Kstar and  
 485 WIHW-Kstar are more accurate in predicting the minimum value of scour depth (Fig 8c). Also,  
 486 none of the empirical equations are able to predict accurately the scour depth.





510 Fig 8. Violin plot of predicted Vs. measured scour depth prediction around (a) vertical, (b) semicircular  
 511 and (c) 45° wing wall abutment.

512 The detailed results of the quantitative metrics for different abutment shape are showed on Table  
 513 5. The hybrid algorithm of WIHW-Kstar outperformed other algorithms in scour depth  
 514 prediction for the vertical abutment shape (RMSE=0.084, RRMSE=4.081%, NSE=0.985,  
 515 WI=0.996 and LM=0.857) followed by RC-Kstar (RMSE=0.097, RRMSE=4.711%, NSE=0.980,



516 WI=0.994 and LM=0.850), RS-Kstar (RMSE=0.124, RRMSE=5.998%, NSE=0.968, WI=0.991  
517 and LM=0.823), DA-Kstar (RMSE=0.111, RRMSE=5.363%, NSE=0.974, WI=0.993 and  
518 LM=0.830), BA-Kstar (RMSE=0.123, RRMSE=5.941%, NSE=0.968, WI=0.991 and  
519 LM=0.823), Kstar (RMSE=0.164, RRMSE=7.910%, NSE=0.944, WI=0.984 and LM=0.804),  
520 Dey and Barbhuiya (RMSE=0.522, RRMSE=25.125%, NSE=0.319, WI=0.883 and LM=-0.037)  
521 and Muzzammil (2010) (RMSE=4.618, RRMSE=222%, NSE=-42.375, WI=0.257 and LM=-  
522 7.764). Overall, all standalone and hybrid algorithms obtained excellent performance ( $RRMSE \leq$   
523 10%), Dey and Barbhuiya (2005) had a fair performance ( $20\% \leq RRMSE \leq 30\%$ ) while  
524 Muzzammil (2010) performed rather poorly (Despotovic et al, 2016). According to NSE, all  
525 standalone and hybrid algorithms had a very good performance ( $NSE > 0.75$ ) while Dey and  
526 Barbhuiya (2005) and Muzzammil (2010) performed rather poorly (Moriassi et al., 2007). Also,  
527 based on the RMSE metric BA, DA, RC, RS and WIHW could enhance the predictive power of  
528 the standalone algorithms of about 25%, 32.3%, 40.85%, 24.4% and 48.8% respectively. Finally,  
529 results based on the RMSE show that the WIHW-Kstar appears to be most effective model with  
530 about 8384% and 98.17% higher performance than Dey and Barbhuiya (2005) and Muzzammil  
531 (2010), respectively. According to our results, the RC-Kstar models outperforms all other  
532 algorithm for both semicircular and 45° Wing abutment shape while Dey and Barbhuiya (2005)  
533 and Muzzammil (2010) had the lower performance. According to the RRMSE and NSE metrics,  
534 all standalone and hybrid algorithms had an excellent performance for both semicircular and 45°  
535 Wing abutment.

536 It is interesting to note how BA, DA, RC, RS and WIHW could enhance the prediction power of  
537 the standalone algorithms of about 3.93%, 14.96%, 30.70%, 8.66% and 20.47% respectively for  
538 semicircular abutment (based on the RMSE metric). Also, the RC-Kstar is the most effective

539 model and has about 72.92% and 97.37% higher performance than while Dey and Barbhuiya  
 540 (2005) and Muzzammil (2010), respectively (based on the RMSE metric). According to the  
 541 results of RMSE, BA, DA, RC, RS and WIHW could enhance the prediction power of the  
 542 standalone algorithms of about 3.97%, 21.85%, 32.45%, 20.52% and 26.49%, respectively, for  
 543 45° wing abutment. Also, the result of the RC-Kstar is the most effective model, and has about  
 544 82.07% and 97.46% higher performance than while Dey and Barbhuiya (2005) and Muzzammil  
 545 (2010), respectively.

546 **Table 5.** Quantitative performance of the tested models

Abutment shape	Models	RMSE	RRMSE (%)	NSE	WI	LM
Vertical	Kstar	0.164	7.91	0.944	0.984	0.804
	BA-Kstar	0.123	5.941	0.968	0.991	0.823
	DA-Kstar	0.111	5.363	0.974	0.993	0.830
	RC-Kstar	0.097	4.711	0.98	0.994	0.850
	RS-Kstar	0.124	5.998	0.968	0.991	0.823
	WIWH-Kstar	0.084	4.081	0.985	0.996	0.857
	Dey and Barbhuiya (2005)	0.522	25.125	0.319	0.883	-0.037
	Muzzammil (2010)	4.618	222	-42.375	0.257	-7.764
Semicircular	Kstar	0.127	8.069	0.954	0.987	0.775
	BA-Kstar	0.122	7.762	0.957	0.988	0.789
	DA-Kstar	0.108	6.913	0.966	0.99	0.806
	RC-Kstar	0.088	5.603	0.977	0.994	0.854
	RS-Kstar	0.116	7.365	0.961	0.989	0.799
	WIWH-Kstar	0.101	6.463	0.97	0.992	0.824
	Dey and Barbhuiya (2005)	0.325	20.668	0.698	0.938	0.435
	Muzzammil (2010)	3.355	213.036	-30.993	0.286	-6.145
45° Wing	Kstar	0.151	8.692	0.937	0.982	0.763
	BA-Kstar	0.145	8.319	0.942	0.983	0.777
	DA-Kstar	0.118	6.783	0.961	0.989	0.804
	RC-Kstar	0.102	5.853	0.971	0.992	0.833
	RS-Kstar	0.120	6.903	0.96	0.989	0.797
	WIWH-Kstar	0.111	6.38	0.966	0.99	0.813
	Dey and Barbhuiya (2005)	0.569	32.636	0.115	0.847	-0.01
	Muzzammil (2010)	4.012	229.88	-42.877	0.254	-7.018

547

548 The Kstar algorithm is able to match the observed values obtained in the flume experiments by  
549 Dey and Barbhuiya (2005) with higher performance than traditional empirical approaches and  
550 the other tested methods. Being based on heuristic search, the Kstar algorithm leads to  
551 considerable improvements in terms of both memory and runtime (Aljazzar and Leue, 2011).  
552 Because of the higher flexibility of the structures of the ensemble algorithms, in most of cases,  
553 these models show better performance than standalone algorithms (De'Ath and Fabricius, 2000).

554 Determining the proper combination of input parameters is one of the most important steps in  
555 developing a precise AI model. We investigated the combinations using a sensitivity analysis to  
556 find the most significant input variables to estimate the scour depth.  $F_e$  resulted one of the most  
557 important parameters in estimating scour depth for three shapes of abutments, probably because  
558 the flow velocity is embedded in the  $F_e$  parameter. Garde et al. (1961) noted that the optimum  
559 scour depth happens at the condition of threshold for the movement of sediments and that the  
560 flow rate defined in excess of Froude number has a major impact on the depth of the scour. In  
561 addition, Froehlich (1989) showed that the excess Froude influence the scour depth in a uniform  
562 sediment bed, under clear-water condition, and our results are in agreement with that. The other  
563 AI models proposed by Azimi et al. (2019), Bonakdari et al. (2020), Moradi et al. (2019) and  
564 Najafzadeh et al. (2013b) all mentioned that the influence of  $F_e$  parameter in the prediction of  
565 scour depth is remarkable.

566 By using the same dataset of Dey and Barbhuiya (2005) different researchers attempted to  
567 estimate scour depth by employing other AI approaches (Azimi et al., 2019; Bonakdari et al.,  
568 2020; Moradi et al., 2019). Moradi et al. (2019) used a neuro-fuzzy-embedded subtractive  
569 clustering (ANFIS–SC) method to forecast scour depth and their results showed that ANFIS-SC  
570 with RMSE of 0.154 scored the better performance in estimating the scour depth. The

571 application of ELM method in predicting scour depth was investigated by Bonakdari et al.  
572 (2020) that showed that the best ELM model can predict scour depth with RMSE of 0.177. In  
573 another research, Azimi et al., (2019) studied an application of a hybrid model as ANFIS-  
574 GA/SVD to predict scour depth and they concluded that their proposed model is robust in  
575 estimating scour depth with RMSE of 0.135. In our study we used a different ensemble-based  
576 models to the predict scour depth for each shape of abutments. For vertical-wall abutments the  
577 WIWH-Kstar model showed the best performance among all other models, with a RMSE of  
578 0.084. In semicircular and 45° wing wall, the RC-Kstar (RMSE of 0.088 and 0.102, respectively)  
579 indicated higher accuracy in predicting scour depth. In comparison with other AI methods which  
580 were previously applied by different researchers, our proposed methods are more robust in scour  
581 depth estimations and have a higher prediction power. This is probably due to the fact that the  
582 model structure of our proposed algorithms work better than traditional models and it is better to  
583 model the data of each shape separately in order to avoid having to include in the analysis a  
584 shape factor.

585 The outcomes of the proposed models compared with the scour depth values which calculated  
586 from equations proposed by Dey and Barbhuiya (2005) and Muzzammil (2010) demonstrated  
587 that the proposed WIWH-Kstar and RC-Kstar for vertical wall, and semicircular and 45° wing  
588 wall respectively, predict more precisely the experimental scours than empirical approaches.

589 One of the main sources of uncertainty in our study refers to the extent of the dataset. Each set of  
590 data was about 100 raw data. In this sense, any further experimental dataset would allow to  
591 refine the modeling efforts. It is also worth pointing out that it would be recommendable to select  
592 dataset from several researchers to build a training dataset for algorithms. Also, a more extended

593 dataset would allow a proper calibration and validation, and ideally a validation performed with  
594 data obtained in a different experimental setting.

## 595 **5. Conclusion**

596 Inaccurate predictions of scour depth ( $ds$ ) at bridge abutment can cause the failure of strategic  
597 structures. Due to the complexity of the scour process with non-linearity structure, simple  
598 empirical equations are not able to predict  $ds$  accurately. In the present study, standalone Kstar  
599 model and five novel hybrid algorithm of bagging (BA), dagging (DA), random committee (RC),  
600 random subspace (RS), and weighted instance handler wrapper (WIHW) (i.e. BA-Kstar, DA-  
601 Kstar, RC-Kstar, RS-Kstar, WIWH-Kstar) were applied for  $ds$  prediction in a clear water (no  
602 sediment feeding) condition and the result were compared with two most common empirical  
603 equations as a benchmark. The results of the current study are the following:

- 604 • According to Pearson correlation coefficient, the  $F_e$  parameter has the highest effect on  
605 the scour depth prediction at each shape of Vertical – wall, Semicircular and 45° wing –  
606 wall, followed by  $d_{50}/l$ ,  $h/l$  and  $d_{50}/h$ , respectively.
- 607 • For vertical-wall shape abutment, the best input is the combination of  $F_e$ ,  $d_{50}/l$ ,  $h/l$   
608 parameters, and result shows that involving parameter of  $d_{50}/h$  reduces the modeling  
609 prediction power.
- 610 • For semicircular and 45° wing shape abutment, the input combination of  $F_e$ ,  $d_{50}/l$  has the  
611 highest effectiveness.
- 612 • The sensitivity analysis revealed that  $F_e$  has the largest effect on the  $ds$  prediction around  
613 vertical abutment shape, while the  $h/l$  parameter has the great effect on the  $ds$  prediction  
614 around semicircular and 45° wing – wall abutment.

- 615 • The sensitivity analysis revealed that  $F_e$ ,  $d_{50}/l$  and  $h/l$  increase the predictive power of the  
616 modeling for vertical abutment, while for semicircular and  $45^\circ$  wing – wall abutment  
617 incorporating  $d_{50}/l$  and  $F_e$  and removing  $d_{50}/h$ , and  $h/l$  cause to higher performance.
- 618 • Result showed that hybrid algorithm of WIHW-Kstar outperform of other algorithm for  
619 vertical abutment while RC-Kstar superior in ds prediction around semicircular and  $45^\circ$   
620 wing abutment.
- 621 • According to NSE, all artificial intelligence models have a very good performance while  
622 the two empirical equations available in the literature have a much lower performance.
- 623 • BA, DA, RC, RS and WIHW could enhance prediction power of the standalone  
624 algorithms significantly.

625

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628 Z.S.K; review and editing: KK and L.M. All authors have read and agreed to the submitted  
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## 631 **References**

- 632 Abd El-Hady Rady, R., 2020. Prediction of local scour around bridge piers: artificial-intelligence-based  
633 modeling versus conventional regression methods. *Appl. Water Sci.* 10, 57.  
634 <https://doi.org/10.1007/s13201-020-1140-4>
- 635 Ahmad, M.W., Reynolds, J., Rezgui, Y., 2018. Predictive modelling for solar thermal energy systems: A  
636 comparison of support vector regression, random forest, extra trees and regression trees. *J. Clean.*  
637 *Prod.* 203, 810–821. <https://doi.org/10.1016/j.jclepro.2018.08.207>
- 638 Aljazzar, H., Leue, S., 2011. K\*: A heuristic search algorithm for finding the k shortest paths. *Artif.*  
639 *Intell.* 175, 2129–2154. <https://doi.org/10.1016/j.artint.2011.07.003>
- 640 Amini, A., Melville, B.W., Ali, T.M., Ghazali, A.H., 2012. Clear-water local scour around pile groups in  
641 shallow-water flow. *J. Hydraul. Eng.* 138, 177–185. [https://doi.org/10.1061/\(ASCE\)HY.1943-](https://doi.org/10.1061/(ASCE)HY.1943-7900.0000488)  
642 [7900.0000488](https://doi.org/10.1061/(ASCE)HY.1943-7900.0000488)

- 643 Ataie-Ashtiani, B., Baratian-Ghorghi, Z., Beheshti, A.A., 2010. Experimental investigation of clear-water  
644 local scour of compound piers. *J. Hydraul. Eng.* 136, 343–351.  
645 [https://doi.org/10.1061/\(ASCE\)0733-9429\(2010\)136:6\(343\)](https://doi.org/10.1061/(ASCE)0733-9429(2010)136:6(343))
- 646 Ataie-Ashtiani, B., Beheshti, A.A., 2006. Experimental investigation of clear-water local scour at pile  
647 groups. *J. Hydraul. Eng.* 132, 1100–1104. [https://doi.org/10.1061/\(ASCE\)0733-](https://doi.org/10.1061/(ASCE)0733-9429(2006)132:10(1100))  
648 [9429\(2006\)132:10\(1100\)](https://doi.org/10.1061/(ASCE)0733-9429(2006)132:10(1100))
- 649 Azamathulla, H.M., 2012. Gene-expression programming to predict scour at a bridge abutment. *J.*  
650 *Hydroinformatics* 14, 324–331. <https://doi.org/10.2166/hydro.2011.135>
- 651 Azamathulla, H.M., Ghani, A.A., Zakaria, N.A., 2009. ANFIS-based approach to predicting scour  
652 location of spillway. *Proc. Inst. Civ. Eng. Water Manag.* 162, 399–407.  
653 <https://doi.org/10.1680/wama.2009.162.6.399>
- 654 Azimi, H., Bonakdari, H., Ebtehaj, I., ... S.T.-F.S. and, 2017, undefined, n.d. Evolutionary Pareto  
655 optimization of an ANFIS network for modeling scour at pile groups in clear water condition.  
656 Elsevier.
- 657 Azimi, H., Bonakdari, H., Ebtehaj, I., Shabanlou, S., Ashraf Talesh, S.H., Jamali, A., 2019. A pareto  
658 design of evolutionary hybrid optimization of ANFIS model in prediction abutment scour depth.  
659 *Sadhana - Acad. Proc. Eng. Sci.* 44. <https://doi.org/10.1007/s12046-019-1153-6>
- 660 Barbhuiya, A.K., 2003. Clear-water scour at abutments. Indian Institute of Technology, Kharagpur, India.
- 661 Bateni, S.M., Jeng, D.S., 2007. Estimation of pile group scour using adaptive neuro-fuzzy approach.  
662 *Ocean Eng.* 34, 1344–1354. <https://doi.org/10.1016/j.oceaneng.2006.07.003>
- 663 Bonakdari, H., Moradi, F., Ebtehaj, I., Gharabaghi, B., Sattar, A.A., Azimi, A.H., Radecki-Pawlik, A.,  
664 2020. A non-tuned machine learning technique for abutment scour depth in clear water condition.  
665 *Water (Switzerland)* 12, 301. <https://doi.org/10.3390/w12010301>
- 666 Breiman, L., 1996. Bagging predictors. *Mach. Learn.* 24, 123–140.
- 667 Bui, Duie Tien, Khosravi, K., Tiefenbacher, J., Nguyen, H., Kazakis, N., 2020. Improving prediction of  
668 water quality indices using novel hybrid machine-learning algorithms. *Sci. Total Environ.* 721.  
669 <https://doi.org/10.1016/j.scitotenv.2020.137612>
- 670 Cardoso, A.H., Bettess, R., 1999. Effects of time and channel geometry on scour at bridge abutments. *J.*  
671 *Hydraul. Eng.* 125, 388–398. [https://doi.org/10.1061/\(asce\)0733-9429\(1999\)125:4\(388\)](https://doi.org/10.1061/(asce)0733-9429(1999)125:4(388))
- 672 Choubin, B., Darabi, H., Rahmati, O., Sajedi-Hosseini, F., Kløve, B., 2018. River suspended sediment  
673 modelling using the CART model: A comparative study of machine learning techniques. *Sci. Total*  
674 *Environ.* 615, 272–281. <https://doi.org/10.1016/j.scitotenv.2017.09.293>
- 675 Cleary, J.G., Trigg, L.E., 1995. K\*: An Instance-based Learner Using an Entropic Distance Measure, in:  
676 *Machine Learning Proceedings 1995*. pp. 108–114. [https://doi.org/10.1016/b978-1-55860-377-](https://doi.org/10.1016/b978-1-55860-377-6.50022-0)  
677 [6.50022-0](https://doi.org/10.1016/b978-1-55860-377-6.50022-0)
- 678 Coleman, S.E., Lauchlan, C.S., Melville, B.W., 2003. Développement de l'affouillement en eau claire aux  
679 butées de pont. *J. Hydraul. Res.* 41, 521–531. <https://doi.org/10.1080/00221680309499997>
- 680 De'Ath, G., Fabricius, K.E., 2000. Classification and regression trees: A powerful yet simple technique  
681 for ecological data analysis. *Ecology* 81, 3178–3192. <https://doi.org/10.1890/0012->

682 9658(2000)081[3178:CARTAP]2.0.CO;2

683 Dey, S., 1997. Local Scour at Piers, part I: A Review of Development of Research. *Int. J. Sediment Res.*  
684 12, 23–44.

685 Dey, S., Barbhuiya, A.K., 2005. Time Variation of Scour at Abutments. *J. Hydraul. Eng.* 131, 11–23.  
686 [https://doi.org/10.1061/\(ASCE\)0733-9429\(2005\)131:1\(11\)](https://doi.org/10.1061/(ASCE)0733-9429(2005)131:1(11))

687 Dey, S., Barbhuiya, A.K., 2004. Clear-water scour at abutments in thinly armored beds. *J. Hydraul. Eng.*  
688 130, 622–634. [https://doi.org/10.1061/\(ASCE\)0733-9429\(2004\)130:7\(622\)](https://doi.org/10.1061/(ASCE)0733-9429(2004)130:7(622))

689 Dey, S., Lambert, M.F., 2005. Reynolds Stress and Bed Shear in Nonuniform Unsteady Open-Channel  
690 Flow. *J. Hydraul. Eng.* 131, 610–614. [https://doi.org/10.1061/\(ASCE\)0733-9429\(2005\)131:7\(610\)](https://doi.org/10.1061/(ASCE)0733-9429(2005)131:7(610))

691 Dietterich, T.G., 2000. An experimental comparison of three methods for constructing ensembles of  
692 decision trees: bagging, boosting, and randomization. *Mach. Learn.* 40, 139–157.  
693 <https://doi.org/10.1023/A:1007607513941>

694 Ebtehaj, I., Bonakdari, H., Moradi, F., Gharabaghi, B., Khozani, Z.S., Sheikh Khozani, Z., 2018. An  
695 integrated framework of Extreme Learning Machines for predicting scour at pile groups in clear  
696 water condition. *Coast. Eng.* 135, 1–15. <https://doi.org/10.1016/j.coastaleng.2017.12.012>

697 Firat, M., Gungor, M., 2009. Generalized Regression Neural Networks and Feed Forward Neural  
698 Networks for prediction of scour depth around bridge piers. *Adv. Eng. Softw.* 40, 731–737.  
699 <https://doi.org/10.1016/j.advengsoft.2008.12.001>

700 Froehlich, D.C., 1989. Local scour at bridge abutments, in: *Proceedings of the National Conference on*  
701 *Hydraulic Engineering*. New Orleans, USA, pp. 13–18.

702 Garde, R.J., Subramanya, K.S. Nambudripad, K.D., 1961. Study of scour around spur-dikes. *J. Hydraul.*  
703 *Div.* 87, 23–37.

704 Gazi, A.H., Afzal, M.S., Dey, S., 2019. Scour around piers underwaves: Current status of research and its  
705 future prospect. *Water (Switzerland)*. <https://doi.org/10.3390/w11112212>

706 Guven, A., Azamathulla, H.M., 2012. A comparative study of predicting scour around a circular pile. *ICE*  
707 *Marit. Eng.* 165, 31–40.

708 Guven, A., Gunal, M., 2008. Genetic programming approach for prediction of local scour downstream of  
709 hydraulic structures. *J. Irrig. Drain. Eng.* 134, 241–249. [https://doi.org/10.1061/\(ASCE\)0733-9437\(2008\)134:2\(241\)](https://doi.org/10.1061/(ASCE)0733-9437(2008)134:2(241))

711 Hamidi, A., Siadatmousavi, S.M., 2018. Numerical simulation of scour and flow field for different  
712 arrangements of two piers using SSIIM model. *Ain Shams Eng. J.* 9, 2415–2426.  
713 <https://doi.org/10.1016/j.asej.2017.03.012>

714 Harikumar V, S.T., M., Kumar, A., K, A.P., Michael, P., P, A.P., E, N., 2018. Modelling of Scour Depth  
715 Around Bridge Piers using Artificial Neural Network (ANN), *International Journal of Engineering*  
716 *Research & Technology*. *IJERT-International Journal of Engineering Research & Technology*.

717 Ho, T.K., 1998. The random subspace method for constructing decision forests. *IEEE Trans. Pattern*  
718 *Anal. Mach. Intell.* 20, 832–844. <https://doi.org/10.1109/34.709601>

719 Hooshyaripor, F., Tahershamsi, A., 2013. Comparing the Performance of Neural Networks for Predicting



- 720 Peak Outflow from Breached Embankments when Back Propagation Algorithms Meet Evolutionary  
721 Algorithms. *Int. J. Hydraul. Eng.* 1, 55–67. <https://doi.org/10.5923/j.ijhe.20120106.01>
- 722 Hooshyaripor, F., Tahershamsi, A., Golian, S., 2014. Application of copula method and neural networks  
723 for predicting peak outflow from breached embankments. *J. Hydro-Environment Res.* 8, 292–303.  
724 <https://doi.org/10.1016/j.jher.2013.11.004>
- 725 Hosseini, K., Karami, H., Hosseinjanzadeh, H., Ardeshir, A., 2016. Prediction of time-varying maximum  
726 scour depth around short abutments using soft computing methodologies - A comparative study.  
727 *KSCE J. Civ. Eng.* 20, 2070–2081. <https://doi.org/10.1007/s12205-015-0115-8>
- 728 Jahangirzadeh, A., Bassar, H., Akib, S., Karami, H., Naji, S., Shamshirband, S., 2014. Experimental and  
729 numerical investigation of the effect of different shapes of collars on the reduction of scour around a  
730 single bridge pier. *PLoS One* 9, e98592. <https://doi.org/10.1371/journal.pone.0098592>
- 731 Kandasamy, I.K., 1989. Abutment scour. Auckland, New Zealand.
- 732 Kaya, A., 2010. Artificial neural network study of observed pattern of scour depth around bridge piers.  
733 *Comput. Geotech.* 37, 413–418. <https://doi.org/10.1016/j.compgeo.2009.10.003>
- 734 Khosravi, K., Mao, L., Kisi, O., Yaseen, Z.M., Shahid, S., 2018. Quantifying hourly suspended sediment  
735 load using data mining models: Case study of a glacierized Andean catchment in Chile. *J. Hydrol.*  
736 567, 165–179. <https://doi.org/10.1016/j.jhydrol.2018.10.015>
- 737 Khosronejad, A., Diplas, P., Angelidis, D., Zhang, Z., Heydari, N., Sotiropoulos, F., 2020. Scour depth  
738 prediction at the base of longitudinal walls: a combined experimental, numerical, and field study.  
739 *Environ. Fluid Mech.* 20, 459–478. <https://doi.org/10.1007/s10652-019-09704-x>
- 740 Kim, H.S., Nabi, M., Kimura, I., Shimizu, Y., 2014. Numerical investigation of local scour at two  
741 adjacent cylinders. *Adv. Water Resour.* 70, 131–147.  
742 <https://doi.org/10.1016/j.advwatres.2014.04.018>
- 743 Kothyari, U.C., Garde, R.C.J., Ranga Raju, K.G., 1992. Temporal Variation of Scour Around Circular  
744 Bridge Piers. *J. Hydraul. Eng.* 118, 1091–1106. [https://doi.org/10.1061/\(ASCE\)0733-9429\(1992\)118:8\(1091\)](https://doi.org/10.1061/(ASCE)0733-9429(1992)118:8(1091))
- 746 Kotsiantis, S., 2011. Combining bagging, boosting, rotation forest and random subspace methods. *Artif.*  
747 *Intell. Rev.* 35, 223–240. <https://doi.org/10.1007/s10462-010-9192-8>
- 748 Kwan, T., 1988. A study of abutment scour. Auckland.
- 749 Melville, B.W., Raudkivi, A.J., 1977. Caractéristiques de l'écoulement dans des conditions  
750 d'affouillement localisées au voisinage immédiat des piles d'un pont. *J. Hydraul. Res.* 15, 373–380.  
751 <https://doi.org/10.1080/00221687709499641>
- 752 Melville, B.W., Sutherland, A.J., 1988. Design method for local scour at bridge piers. *J. Hydraul. Eng.*  
753 114, 1210–1226. [https://doi.org/10.1061/\(ASCE\)0733-9429\(1988\)114:10\(1210\)](https://doi.org/10.1061/(ASCE)0733-9429(1988)114:10(1210))
- 754 Mohammadpour, R., Ghani, A.A., Azamathulla, H.M., 2013. Estimation of dimension and time variation  
755 of local scour at short abutment. *Int. J. River Basin Manag.*  
756 <https://doi.org/10.1080/15715124.2013.772522>
- 757 Moonen, P., Allegrini, J., 2015. Employing statistical model emulation as a surrogate for CFD. *Environ.*  
758 *Model. Softw.* 72, 77–91. <https://doi.org/10.1016/j.envsoft.2015.06.007>

- 759 Moradi, F., Bonakdari, H., Kisi, O., Ebtehaj, I., Shiri, J., Gharabaghi, B., 2019. Abutment scour depth  
760 modeling using neuro-fuzzy-embedded techniques. *Mar. Georesources Geotechnol.* 37, 190–200.  
761 <https://doi.org/10.1080/1064119X.2017.1420113>
- 762 Moriasi, D.N., Arnold, J.G.J.G.J.G., Van Liew, M.W.M.W.M.W., Bingner, R.L., Harmel, R.D.D., Veith,  
763 T.L.T.L.T.L.T.L., Binger, R.L., Harmel, R.D.D., Veith, T.L.T.L.T.L.T.L., Bingner, R.L., Harmel,  
764 R.D.D., Veith, T.L.T.L.T.L.T.L., 2007. Model evaluation guidelines for systematic quantification of  
765 accuracy in watershed simulations. *Trans. ASABE* 50, 885–900.  
766 <https://doi.org/10.13031/2013.23153>
- 767 Muzzammil, M., 2010. ANFIS approach to the scour depth prediction at a bridge abutment. *J.*  
768 *Hydroinformatics* 12, 474–485. <https://doi.org/10.2166/hydro.2010.004>
- 769 Muzzammil, M., 2008. Application of neural networks to scour depth prediction at the bridge abutments.  
770 *Eng. Appl. Comput. Fluid Mech.* 2, 30–40. <https://doi.org/10.1080/19942060.2008.11015209>
- 771 Najafzadeh, M., Balf, M.R., Rashedi, E., 2016. Prediction of maximum scour depth around piers with  
772 debris accumulation using EPR, MT, and GEP models. *J. Hydroinformatics* 18, 867–884.  
773 <https://doi.org/10.2166/hydro.2016.212>
- 774 Najafzadeh, M., Barani, G.A., Hessami Kermani, M.R., 2013a. GMDH based back propagation algorithm  
775 to predict abutment scour in cohesive soils. *Ocean Eng.* 59, 100–106.  
776 <https://doi.org/10.1016/j.oceaneng.2012.12.006>
- 777 Najafzadeh, M., Barani, G.A., Kermani, M.R.H., 2013b. Abutment scour in clear-water and live-bed  
778 conditions by GMDH network. *Water Sci. Technol.* 67, 1121–1128.  
779 <https://doi.org/10.2166/wst.2013.670>
- 780 Najafzadeh, M., Lim, S.Y., 2015. Application of improved neuro-fuzzy GMDH to predict scour depth at  
781 sluice gates. *Earth Sci. Informatics* 8, 187–196. <https://doi.org/10.1007/s12145-014-0144-8>
- 782 Namaee, M.R., Sui, J., 2019. Impact of armour layer on the depth of scour hole around side-by-side  
783 bridge piers under ice-covered flow condition. *J. Hydrol. Hydromechanics* 67, 240–251.  
784 <https://doi.org/10.2478/johh-2019-0010>
- 785 Nourani, V., Komasi, M., Alami, M.T., 2012. Hybrid Wavelet-Genetic Programming Approach to  
786 Optimize ANN Modeling of Rainfall-Runoff Process. *J. Hydrol. Eng.* 17, 724–741.  
787 [https://doi.org/10.1061/\(ASCE\)HE.1943-5584.0000506](https://doi.org/10.1061/(ASCE)HE.1943-5584.0000506)
- 788 Oliveto, G., Hager, W.H., 2002. Temporal evolution of clear-water pier and abutment scour. *J. Hydraul.*  
789 *Eng.* 128, 811–820. [https://doi.org/10.1061/\(ASCE\)0733-9429\(2002\)128:9\(811\)](https://doi.org/10.1061/(ASCE)0733-9429(2002)128:9(811))
- 790 Omara, H., Elsayed, S.M., Abdealaal, G.M., Abd-Elhamid, H.F., Tawfik, A., 2019. Hydromorphological  
791 Numerical Model of the Local Scour Process Around Bridge Piers. *Arab. J. Sci. Eng.* 44, 4183–  
792 4199. <https://doi.org/10.1007/s13369-018-3359-z>
- 793 Oza, N.C., 2005. Online bagging and boosting, in: *Conference Proceedings - IEEE International*  
794 *Conference on Systems, Man and Cybernetics.* pp. 2340–2345.  
795 <https://doi.org/10.1109/icsmc.2005.1571498>
- 796 Parsaie, A., Haghiabi, A.H., Moradinejad, A., 2019. Prediction of Scour Depth below River Pipeline  
797 using Support Vector Machine. *KSCE J. Civ. Eng.* 23, 2503–2513. <https://doi.org/10.1007/s12205-019-1327-0>

799 Raudkivi, A.J., Ettema, R., 1983. Clear-water scour at cylindrical piers. *J. Hydraul. Eng.* 109, 338–350.  
800 [https://doi.org/10.1061/\(ASCE\)0733-9429\(1983\)109:3\(338\)](https://doi.org/10.1061/(ASCE)0733-9429(1983)109:3(338))

801 Sheikh Khozani, Z., Khosravi, K., Pham, B.T., Kløve, B., Wan Mohtar, W.H.M., Yaseen, Z.M., 2019.  
802 Determination of compound channel apparent shear stress: application of novel data mining models.  
803 *J. Hydroinformatics* 21, 798–811. <https://doi.org/10.2166/hydro.2019.037>

804 Sheppard, D.M., Odeh, M., Glasser, T., 2004. Large scale clear-water local pier scour experiments. *J.*  
805 *Hydraul. Eng.* 130, 957–963. [https://doi.org/10.1061/\(ASCE\)0733-9429\(2004\)130:10\(957\)](https://doi.org/10.1061/(ASCE)0733-9429(2004)130:10(957))

806 Sihag, P., Singh, B., Sepah Vand, A., Mehdipour, V., 2020. Modeling the infiltration process with soft  
807 computing techniques. *ISH J. Hydraul. Eng.* 26, 138–152.  
808 <https://doi.org/10.1080/09715010.2018.1464408>

809 Singh, R.K., Pandey, M., Pu, J.H., Pasupuleti, S., Villuri, V.G.K., 2020. Experimental study of clear-  
810 water contraction scour. *Water Supply* 20, 943–952. <https://doi.org/10.2166/ws.2020.014>

811 Sun, S., Zhang, C., 2007. The selective random subspace predictor for traffic flow forecasting. *IEEE*  
812 *Trans. Intell. Transp. Syst.* 8, 367–373. <https://doi.org/10.1109/TITS.2006.888603>

813 Ting, K.M., Witten, I.H., 1997. Stacking bagged and dagged models. *Proc. of ICML'97* 367–375.

814 Török, G.T., Baranya, S., Rütther, N., Spiller, S., 2014. Laboratory analysis of armor layer development in  
815 a local scour around a groin, in: *Proceedings of the International Conference on Fluvial Hydraulics,*  
816 *RIVER FLOW 2014.* pp. 1455–1462. <https://doi.org/10.1201/b17133-194>

817 Wong, W., 1982. *Scour at bridge abutments.* New Zealand.

818 Yan, T., Chen, L., Xu, M., Zhou, M., 2012. Siphon pipeline resistance characteristic research, in:  
819 *Procedia Engineering.* pp. 99–104. <https://doi.org/10.1016/j.proeng.2012.01.689>

820 Yang, F., Liu, S.H., Wu, Y.L., Tang, X.L., 2005. Lattice Boltzmann subgrid model for lid-driven cavity  
821 flow. *J. Hydrodyn.* 17, 289–294. <https://doi.org/10.1115/HT-FED2004-56100>

822 Yang, Y., Melville, B.W., Macky, G.H., Shamseldin, A.Y., 2020. Experimental study on local scour at  
823 complex bridge pier under combined waves and current. *Coast. Eng.* 160, 103730.  
824 <https://doi.org/10.1016/j.coastaleng.2020.103730>

825 Yaseen, Z.M., Ebtehaj, I., Bonakdari, H., Deo, R.C., Danandeh Mehr, A., Mohtar, W.H.M.W., Diop, L.,  
826 El-shafie, A., Singh, V.P., 2017. Novel approach for streamflow forecasting using a hybrid ANFIS-  
827 FFA model. *J. Hydrol.* 554, 263–276. <https://doi.org/10.1016/j.jhydrol.2017.09.007>

828 Yazdandoost, F., Birgani, Y.T., 2011. Bridge pier scour depth prediction using Artificial Neural  
829 Networks, in: *The 7th IAHR Symposium on River, Coastal and Estuarine Morphodynamics.*  
830 *Beijing, China.*

831

832

833